# Designed to Outperform Traditional Lumber 

LP® SolidStart ${ }^{\oplus}$ Laminated Veneer Lumber (LVL) is a vast improvement over traditional lumber. Problems that naturally occur as sawn lumber dries - twisting, splitting, checking, crowning and warping - are greatly reduced.

## THE STRENGTH IS IN THE ENGINEERING

LP SolidStart LVL is made from ultrasonically and visually graded veneers arranged in a specific pattern to maximize the strength and stiffness of the veneers and to disperse the naturally occurring characteristics of wood, such as knots, that can weaken a sawn lumber beam. The veneers are then bonded with waterproof adhesives under pressure and heat. LP SolidStart LVL beams are exceptionally strong, solid and straight, making them excellent for most primary load-carrying beam applications.

## LP SolidStart LVL: AVAILABLE GRADES AND SIZES

LP SolidStart LVL is available in two standard grades $2900 \mathrm{~F}_{\mathrm{b}}-2.0 \mathrm{E}$ and $2250 \mathrm{~F}_{\mathrm{b}}-1.5 \mathrm{E}$. It comes in a range of depths and lengths, and is available in standard thicknesses of $1-3 / 4$ " and $3-1 / 2$.' The $2900 F_{b}-2.0 \mathrm{E}$ grade is also available in factory-laminated thicknesses (known as "billet beam") of 5-1/4" and 7" to eliminate the need for field nailing and/or bolting of multiple plies. In addition, a water-resistant coating called SiteCote ${ }^{\text {T" }}$ is available for extra weather protection during construction.

## LIFETIME LIMITED WARRANTY

LP SolidStart Engineered Wood Products are backed by a lifetime limited warranty. Visit LPCorp.com or call 1.888.820.0325 for a copy of the warranty.

## SOFTWARE FOR EASY, RELIABLE DESIGN

Our design/specification software enhances your in-house design capabilities. It offers accurate designs for a wide variety of applications with interfaces for printed output or plotted drawings. Through our distributors, we offer component design review services for designs using LP SolidStart Engineered Wood Products.

## CODE EVALUATION

LP SolidStart Laminated Veneer Lumber has been evaluated by CCMC for compliance with the National Building Code of Canada. Contact your local LP SolidStart Engineered Wood Products distributor or visit LPCorp.com for the most current code reports.

## FRIEND TO THE ENVIRONMENT

LP SolidStart LVL is a building material with built-in environmental benefits. It is made of engineered wood substrate, a renewable resource with a reduced environmental impact. LP uses $\mathrm{SFI}^{\oplus}$ certified forest management and procurement systems, which help ensure wood comes from well managed forests. Raw material procurement targets small, fast growing trees. In LP's manufacturing process, no part of the log goes to waste. And only safe, low formaldehyde-emitting resins are used.

## IMPORTANT NOTES

1. LP SolidStart LVL shall be designed for dry-use conditions only. Dry-use applies to products installed in dry, covered and well ventilated interior conditions in which the equivalent average moisture content in lumber will not exceed $15 \%$ nor a maximum of $19 \%$.
2. This guide is valid only for LP SolidStart LVL members supporting loads applied parallel to the face of the veneer ("edge" orientation).
3. The tables in this guide meet the design requirements of the National Building Code of Canada for Limit States Design and assume a normal importance category. Ensure that the specified design loads, duration of load increases and deflection limits that you use to select products from this guide are appropriate for your application and comply with local code requirements. If you do not know the correct design criteria and all the loads imposed on the component from all parts of the structure, seek qualified help from the architect, engineer or designer of the structure. Additional reference data on wood construction is available in the form of building codes, code evaluation reports and other design references.
4. The Quick Reference and Uniform Load Resistance (PLF) tables in this guide are only for uniform loads on simple (single) or equal, continuous (multiple) span members as noted in each table. For other conditions such as concentrated loads, unequal spans, etc., contact your LP SolidStar distributor.
5. Beam spans in this guide are typically measured from center-to-center of supports except for door and window headers. A structurally adequate bearing surface under the full width (thickness) of the member must be provided at each support.
6. Minimum bearing length is $1-1 / 2^{\prime \prime}$ (at least one jack stud or cripple is required unless otherwise noted for a specific table. Refer to the Factored Reaction Resistance chart and the notes for each table. Verify local code requirements for minimum bearing.
7. Total load deflections are based on instantaneous loading. Long term deflection (creep) under sustained load has not been considered.
8. Vibration has not been considered in this guide. If LP SolidStart LVL is used asa floor joist, the designer shall perform the required vibration control checks.
9. LP SolidStart LVL is not cambered.
10. Higher grades of LP SolidStart LVL can be substituted for the indicated grade.
11. LP SolidStart LVL sized with the tables and design values in this guide requires continuous lateral restraint of the compression edge. Continuous restraint is defined as a maximum unbraced length of 24." This restraint is normally provided by sheathing and/or other framing members, which shall be adequately anchored to the LVL and the supporting structure. Framing conditions that do not provide continuous lateral restraint require special design. Contact your LP SolidStart Engineered Wood Products distributor. Caution: Failure to provide adequate lateral restraint could result in an unstable member and reduce its load capacity.
12. Lateral restraint shall also be provided at all supports to prevent rotation or twisting.
13. Refer to the Connection Details page for information on designing nailed and bolted connections, minimum nail spacing and end distances, and for properly connecting multiple plies of LVL to form a built-up member.
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SPECIFIED STRENGTHS \& STIFFNESS (PSI)

| Grade | Bending $f_{b}{ }^{4}$ | Modulus of Elasticity ( $\times 10^{6}$ PSI) | Shear $\mathrm{f}_{\mathrm{v}}$ | Compression |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\underset{\text { (Perpendicular To Grain) }}{\mathbf{f}_{\text {cp }}}$ |
| $2900 \mathrm{~F}_{\mathrm{b}}-2.0 \mathrm{E}$ | 5359 | 2.0 | 530 | 5107 | 1365 |

NOTES:

1. LP® SolidStart ${ }^{\ominus}$ LVL shall be designed for dry-use conditions only. Dry-use applies to products installed in dry, covered and well ventilated interior conditions in which the equivalent average moisture content in lumber will not exceed 15\% nor a maximum of 19\%.
2. The specified strengths and stiffness are for standard load duration. Specified strengths shall be adjusted according to code. Stiffness shall not be adjusted
3. The specified strengths and stiffness are for members supporting loads applied parallel to the wide face ("edge" or "beam" orientation).
4. The specified Bending strength, $\mathrm{f}_{\mathrm{b}}$, is tabulated for 12 " depth. For depths greater than 12, , multiply $\mathrm{f}_{\mathrm{b}}$ by ( $12 /$ depth $)^{0.143}$

For depths less than 12," multiply $f_{b}$ by ( 12 /depth $)^{0.117}$. For depths less than $3-1 / 2$," multiply $f_{b}$ by 1.147.
5. Deflection calculations shall include both bending and shear deformations.

Deflection for a simple span, uniform load: $\Delta=\frac{270 \mathrm{wL}^{4}}{\mathrm{End}^{3}}+\frac{28.8 \mathrm{wL}^{2}}{\mathrm{E} \text { 2 }}$ Where: $\Delta=$ deflection (in) $\quad \mathrm{E}=$ modulus of elasticity (from table)
$w=$ uniform load (plf) $\quad b=$ width (in)
$L=$ design span (ft) $\quad d=$ depth (in)
Equations for other conditions can be found in engineering references.
SECTION PROPERTIES AND FACTORED RESISTANCES

| Depth | Weight <br> (lb/ft) |  |  |  | Factored Moment ${ }^{2}$ ( lb -ft) |  |  |  | Factored Shear (Ib) |  |  |  | Moment of Inertia (in ${ }^{4}$ ) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\text {7 }}$ | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\text {7 }}$ | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\text {7 }}$ | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\text {7 }}$ |
| 7-1/4" | 3.6 | 7.3 | 10.9 | 14.5 | 6517 | 13033 | 19550 | 26067 | 4035 | 8069 | 12104 | 16139 | 56 | 111 | 167 | 222 |
| 9-1/4" | 4.6 | 9.3 | 13.9 | 18.5 | 10325 | 20649 | 30974 | 41299 | 5148 | 10295 | 15443 | 20591 | 115 | 231 | 346 | 462 |
| 9-1/2" | 4.8 | 9.5 | 14.3 | 19.0 | 10858 | 21716 | 32574 | 43432 | 5287 | 10574 | 15860 | 21147 | 125 | 250 | 375 | 500 |
| 11-1/4" | 5.6 | 11.3 | 16.9 | 22.5 | 14943 | 29887 | 44830 | 59774 | 6261 | 12521 | 18782 | 25043 | 208 | 415 | 623 | 831 |
| 11-7/8" | 5.9 | 11.9 | 17.8 | 23.8 | 16550 | 33100 | 49651 | 66201 | 6608 | 13217 | 19825 | 26434 | 244 | 488 | 733 | 977 |
| 14 " | 7.0 | 14 | 21.0 | 28.0 | 22476 | 44953 | 67429 | 89905 | 7791 | 15582 | 23373 | 31164 | 400 | 800 | 1201 | 1601 |
| $16 "$ | 8.0 | 16 | 24.0 | 32.0 | 28802 | 57604 | 86406 | 115208 | 8904 | 17808 | 26712 | 35616 | 597 | 1195 | 1792 | 2389 |
| 18" | 9.0 | 18 | 27.0 | 36.1 | 35844 | 71689 | 107533 | 143377 | 10017 | 20034 | 30051 | 40068 | 851 | 1701 | 2552 | 3402 |

NOTES

1. The Factored Moment and Shear are for standard load duration and shall be adjusted according to code.
2. The tabulated Factored Moment Resistance assumes continuous lateral support of the compression edge. For other conditions, multiply the Factored Moment Resistance by the beam lateral stability factor, $\mathrm{K}_{\mathrm{L}}$, as defined in the CSA 086 .
3. The $3-1 / 2$ " $5-1 / 4^{\prime \prime}$ and $7^{\prime \prime}$ beam widths listed above can be either a single piece or a combination of widths. For example, a 7 " wide beam may be a single billet beam of 7 ", two plies of $3-1 / 2$ ", a single $1-3 / 4^{\prime \prime}$ attached to a $5-1 / 4^{\prime \prime}$ billet beam, a $3-1 / 2^{\prime \prime}$ with a $1-3 / 4^{\prime \prime}$ ply attached to each face, or four plies of $1-3 / 4^{\prime \prime}$. Refer to the Connection Assemblies details on page 22 for additional information.
4. The tabulated weight is an estimate and shall only be used for design purposes. Contact LP for actual shipping weights.

## FASTENERS:

Refer to pages 22-23 for information on connecting multiple plies and for the equivalent specific gravity for design of nailed and bolted connections.

## FACTORED REACTION RESISTANCE (LBS)

| Bearing Length |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | 1-1/2" | 2" | 2-1/2" | 3" | 3-1/2" | 4" | 4-1/2" | 5" | 5-1/2" | 6" | 6-1/2" | $7{ }^{\prime \prime}$ | 7-1/2" | 8" | 8-1/2" | 9" | 9-1/2" | 10" | 10-1/2" | 11" | 11-1/2" | 12" |
| 1-3/4" | 2866 | 3822 | 4777 | 5733 | 6688 | 7644 | 8599 | 9555 | 10510 | 11466 | 12421 | 13377 | 14332 | 15288 | 16243 | 17199 | 18154 | 19110 | 20065 | 21021 | 21976 | 22932 |
| 3-1/2" | 5733 | 7644 | 9555 | 11466 | 13377 | 15288 | 17199 | 19110 | 21021 | 22932 | 24843 | 26754 | 28665 | 30576 | 32487 | 34398 | 36309 | 38220 | 40131 | 42042 | 43953 | 45864 |
| 5-1/4" | 8599 | 11466 | 14332 | 17199 | 20065 | 22932 | 25798 | 28665 | 31531 | 34398 | 37264 | 40131 | 42997 | 45864 | 48730 | 51597 | 54463 | 57330 | 60196 | 63063 | 65929 | 68796 |
| $7{ }^{\prime \prime}$ | 11466 | 15288 | 19110 | 22932 | 26754 | 30576 | 34398 | 38220 | 42042 | 45864 | 49686 | 53508 | 57330 | 61152 | 64974 | 68796 | 72618 | 76440 | 80262 | 84084 | 87906 | 91728 |

## NOTES:

1. Tabulated values are based on the factored compression resistance, perpendicular-to-grain, of the LVL. This is suitable for beams bearing on steel or the end-grain of studs.
2. Verify that the support for the beam is structurally adequate to carry the reaction. The compressive resistance, parallel-to-grain, of studs may require more studs than the bearing length above indicates.
3. For beams bearing on wood plates, the required bearing length will increase based on the bearing resistance (compression perpendicular-to-grain) of the species and grade used for the plate material.
4. Verify local code requirements concerning minimum bearing

## FLOOR BEAM QUICK REFERENCE DETAILS (see page 5 for tables)



## TO USE:

1. Select the correct table for the supported floor joist condition (simple or continuous)
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a $16^{\prime}$ span carries $15^{\prime}-0^{\prime \prime}$ simple span joists on each side
SOLUTION: Using the Simple-Span Floor Joists table with 30'-0" span carried, select either $3-1 / 2^{\prime \prime} \times 16^{\prime \prime}$ or $5-1 / 4^{\prime \prime} \times 14$.'
NOTE: The $3-1 / 2^{\prime \prime} \times 16^{\prime \prime}$ requires $7-1 / 2^{\prime \prime}$ intermediate bearing if a multiple span beam.


CONTINUOUS FLOOR JOISTS (SPECIFIED FLOOR LOADS: 40 PSF LIVE LOAD, 15 PSF DEAD LOAD)

| Span | Beam <br> Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $20^{\prime}$ | 22' | $24^{\prime}$ | $26^{\prime}$ | $28^{\prime}$ | $30^{\prime}$ | $32^{\prime}$ | 34' | $36^{1}$ | 38' | $40^{\prime}$ |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" |
| 12'-0' | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" | - |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'-0" | 3-1/2" | 14 " | 14" | 14" | 14" | 14" | 14" | 16 " | - | - | - | - |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | $14^{\prime \prime}$ | 14" | 14" | 14" | $14^{\prime \prime}$ |
| 16'-0" | 3-1/2" | 14" | 16 " | 16" | 16" | 16" | - | - | - | - | - | - |
|  | 5-1/4" | 11-7/8" | 14 " | 14 " | 14 " | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | 14" | $16^{\prime \prime}$ | 16" | 16" |
| 18'-0" | 3-1/2" | $16^{\prime \prime}$ | 16" | 18" | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 14 " | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 16" | 16" | 16" | 18" |
| 20'-0" | 3-1/2" | 18" | 18" | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | 18" | - |
| 22'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
| 24'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18 " | - | - | - | - | - | - | - | - | - |

SIMPLE-SPAN FLOOR JOISTS (SPECIFIED FLOOR LOADS: 40 PSF LIVE LOAD, 15 PSF DEAD LOAD)

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $20^{\prime}$ | 22' | 24' | $26^{1}$ | $28^{\prime}$ | 30' | 32' | 34 | 361 | 38' | 40' |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 12'-0" | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'-0" | 3-1/2" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | $14^{\prime \prime}$ | $14 "$ | $14 "$ | 14" | 14" | 14" | 14" |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 11-7/8" | 11-7/8" |
| 16'-0" | 3-1/2" | 14" | 14 " | $14{ }^{\prime \prime}$ | 14 " | 16 " | 16 " | 16 " | 16" | 16" | 16 " | - |
|  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | 14" |
| 18'-0" | 3-1/2" | $16 "$ | 16 " | $16 "$ | 16" | 16" | 16 " | 18" | 18" | - | - | - |
|  | 5-1/4" | 14 " | 14 " | 14" | 14 " | 14 " | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
| 20'-0" | 3-1/2" | 16 " | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18 " | 18" |
| 22'-0" | 3-1/2" | 18" | 18" | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | 18" | 18" |
| 24'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |

NOTES:

1. Use the Continuous Floor Joists table where the floor joists are continuous (multiple span) over the beam. Use the Simple-Span Floor Joists table where the floor joists frame into the side of or end on top of the beam.
2. Span is center-to-center of supports and is valid for simple and equal, continuous beam spans
3. End supports require 3 " bearing

Interior supports require $6^{\prime \prime}$ bearing except $7-1 / 2^{\prime \prime}$ is required where bold
The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
4. Deflections are limited to $L / 360$ live load and $L / 240$ total load.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages $22-23$ for connection details.
6. Do not use where marked "-"

## LVL 2900F ${ }_{\mathrm{b}}$-2.0E Combined Beam Quick Reference Tables

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a 9'-6" span supports a 32'-0" span carried for a 20 psf Roof Live load. SOLUTION: Using the correct table for the roof load with $32^{\prime}-0^{\prime \prime}$ span carried, select either $3-1 / 2^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ or 5 -1/4" $\times$ 9-1/4.'.




## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require $3^{\prime \prime}$ bearing except $4-1 / 2^{\prime \prime}$ is required where bold. The end supports for the standard garage door spans of $9^{\prime}-6$ ", $16^{\prime}-66^{\prime \prime}$ and $18^{\prime}-6^{\prime \prime}$ are limited to $3^{\prime \prime}$ (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $L / 360$ live/snow load and $L / 240$ total load.
4. Loads include 100 plf for an exterior wall and assume a 2' maximum overhang on the roof and an interior support at mid-span of the floor joists.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages $22-23$ for connection details
6. Do not use where marked "-"

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a 9 '-6" span supports a $32^{\prime}-0$ " span carried for a 40 psf Roof Snow load.
SOLUTION: Using the correct table for the roof load with $32^{\prime}-0^{\prime \prime}$ span carried, select either $3-1 / 2^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ or 5 -1/4" $\times 9-1 / 4$.'


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | $36^{\prime}$ | 38' | $40^{\prime}$ |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | - | - | - |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" |
|  | 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 12'-0" | 3-1/2" | 11-7/8" | 11-7/8" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14" | 14" | 14" | 14" | 14" | 16" | 16" |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14 "$ |
|  | 14'-0" | 3-1/2" | $14 "$ | $14 "$ | 16" | 16" | 16" | 16" | 16" | 16" | 18" | - | - |
|  |  | 5-1/4" | 11-7/8" | 14 " | 14 " | 14 " | 14 " | 14 " | $14 "$ | $14 "$ | $14 "$ | 16" | 16" |
|  | 16'-0" | 3-1/2" | 16" | 16" | 18" | 18" | 18" | 18" | - | - | - | - | - |
|  |  | 5-1/4" | 14 " | 14 " | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 16" | 16" | 16" | 18" | 18" |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 14" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 16" | 16" | - | - | - | - | - | - |
|  | 18'-0" | 3-1/2" | 18" | 18" | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | $16 "$ | $16^{\prime \prime}$ | $18{ }^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | - | - |
|  | 18'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18" | 18" | - | - | - | - | - | - | - |
|  | 20'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 18" | 18" | 18" | - | - | - | - | - | - | - | - |
|  | 22'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |
|  | 24-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |


|  | Span | Beam <br> Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24 | $26^{\prime}$ | 28' | 30' | 32' | $34 \cdot$ | 36 | 38' | $40^{\prime}$ |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-6" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | - | - | - | - | - |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 12'-0" | 3-1/2" | 14" | 14" | 14" | 14" | 14" | 14" | 16" | 16" | 16" | 16" | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14" |
|  | 14'-0" | 3-1/2" | 16" | 16" | 16" | 16" | 16" | 18" | - | - | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 16" | 16" | 16" | 16" | 16" |
|  | 16'-0" | 3-1/2" | 18" | 18" | 18" | 18" | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 16" | 16" | 18" | 18" | 18" | 18" | 18" |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | - | - | - | - | - | - | - |
|  | 18'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
|  | 18'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 18" | 18" | - | - | - | - | - | - | - | - | - |
|  | 20'-0" | 3-1/2" | 号 | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 18" | - | - | - | - | - | - | - | - | - | - |
|  | 22'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |
|  | 24'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require $3^{\prime \prime}$ bearing except $4-1 / 2^{\prime \prime}$ is required where bold. The end supports for the standard garage door spans of $9^{\prime}-6$ ", $16^{\prime}-6$ " and $18^{\prime}-6$ " are limited to $3^{\prime \prime}$ (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads include 100 plf for an exterior wall and assume a 2' maximum overhang on the roof and an interior support at mid-span of the floor joists.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages $22-23$ for connection details.
6. Do not use where marked "-"

## LVL $2900 \mathrm{~F}_{\mathrm{b}}$-2.0E Roof Beam Quick Reference Tables

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a $16^{\prime}-6{ }^{\prime \prime}$ span supports a $36^{\prime}-0^{\prime \prime}$ span carried for a 30 psf Roof Snow load.
SOLUTION: Using the correct table for the roof load with $36^{\prime}-0$ " span carried, select either
$3-1 / 2^{\prime \prime} \times 16^{\prime \prime}$ or $5-1 / 4^{\prime \prime} \times 14$."




## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require 3 " bearing except $4-1 / 2^{\prime \prime}$ is required where bold.

The end supports for the standard garage door spans of $9^{\prime}-66^{\prime \prime}, 16^{\prime}-66^{\prime \prime}$ and $18^{\prime}-66^{\prime \prime}$ are limited to $3^{\prime \prime}$ (two trimmers) on each end.
The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads assume a 2' maximum overhang on the roof.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners.

Refer to pages 22-23 for connection details.
6. Do not use where marked "-".

## LVL 2900F ${ }_{\mathrm{b}}$-2.0E Roof Beam Quick Reference Tables

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a $16^{\prime}-6$ " span supports a $36^{\prime}-0$ " span carried for a 50 psf Roof Snow load.
SOLUTION: Using the correct table for the roof load with $36^{\prime}-0$ " span carried, select a $\mathbf{5 - 1 / 4 " \times 1 6 . "}$
NOTE: A 3-1/2" beam does not work.


| $\begin{gathered} \text { SPECIFIED LOADS } \\ \text { ROOF: } 40 \text { PSF SNOW, } 15 \text { PSF DEAD } \end{gathered}$ | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | 26' | 28' | $30^{\prime}$ | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 12'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | 14 " |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 14'-0" | 3-1/2" | 11-7/8" | 14 " | 14 " | $14 "$ | 14" | 14" | 14" | 14 " | 16 " | 16" | 16" |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
|  | 16'-0" | 3-1/2" | 14" | $14{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | 16" | 16" | 16" | 18" | 18" | 18" |
|  |  | 5-1/4" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  | 16'-6" | 3-1/2" | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | - | - | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14 "$ | 14" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  | 18'-0" | 3-1/2" | $16 "$ | $16^{\prime \prime}$ | 18 " | 18 " | 18" | 18" | 18" | 18" | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18" | 18" |
|  | 18'-6" | 3-1/2" | $16 "$ | 18" | 18 " | 18" | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16 "$ | $16 "$ | $16^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ |
|  | 20'-0" | 3-1/2" | 18 " | 18" | 18" | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $18^{\prime \prime}$ | $18^{\prime \prime}$ | $18^{\prime \prime}$ | 18" | 18" | $18^{\prime \prime}$ | - |
|  | 22'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - | - |
|  | 24'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 18" | - | - | - | - | - | - | - | - | - | - |


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30 | $32^{\prime}$ | 34' | $36^{\prime}$ | 38' | $40^{\prime}$ |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" |
|  | 10'-0" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" |
|  | 12'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14" | 14" | 14" | $14 "$ | 14" | 14" |
|  |  | 5-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" |
|  | 14'-0" | 3-1/2" | $14 "$ | $14 "$ | $14 "$ | $14 "$ | 16 " | 16 " | 16" | 16" | 16" | 16" | 16" |
|  |  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14^{\prime \prime}$ |
|  | 16'-0" | 3-1/2" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | 18" | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  | 16'-6" | 3-1/2" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | - |
|  | 18'-0" | 3-1/2" | 18" | 18" | 18" | 18" | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | 18" |
|  | 18'-6" | 3-1/2" | 18" | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $18^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18^{\prime \prime}$ | - | - | - |
|  | 20'-0" | 3-1/2" | - | - | , | , | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
|  | 22'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 18" | 18" | - | - | - | - | - | - | - | - | - |
|  | 24'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require 3 " bearing except $4-1 / 2^{\prime \prime}$ is required where bold

The end supports for the standard garage door spans of $9^{\prime \prime}-6$ ", $16^{\prime}-66^{\prime \prime}$ and $18^{\prime}-66^{\prime \prime}$ are limited to $3^{\prime \prime}$ (two trimmers) on each end.
The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads assume a 2 ' maximum overhang on the roof.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners.

Refer to pages 22-23 for connection details.
6. Do not use where marked "-".

## TO USE:

1. Select the span required.
2. Divide the design loads by the number of plies to verify each ply of the member. Divide the design loads by 2 to verify a $3-1 / 2^{\prime \prime}$ width or by 3 to verify a $5-1 / 4^{\prime \prime}$ width
3. Compare the factored design total load to the Factored Total Resistance column
4. Compare the unfactored design total load to the Total Load Deflection Resistance.
5. Compare the unfactored design live load to the Live Load Deflection Resistance for the appropriate deflection limit.
6. Select a product that satisfies all three conditions

## EXAMPLE:

For a 16 ' beam span, select a 2- and 3-ply beam that satisfies an L/360 Live Load deflection limit for the following specified loads: Live Load = 480 plf; Dead Load $=180 \mathrm{plf}$

## CALCULATE DESIGN LOADS

Factored Total Load $=(1.5 \times 480)+(1.25 \times 180)=945 \mathrm{plf}$
Unfactored Total Load $=480+180=660 \mathrm{plf}$

## SOLUTION FOR A 2-PLY BEAM:

Factored Total Load per ply $=945 / 2=473$ plf Unfactored Total Load per ply $=660 / 2=330$ plf Unfactored Live Load per ply $=480 / 2=240$ plf Use 2 plies 1-3/4" x 14"

## SOLUTION FOR A 3-PLY BEAM:

Factored Total Load per ply $=945 / 3=315 \mathrm{plf}$ Unfactored Total Load per ply $=660 / 3=220$ plf Unfactored Live Load per ply $=480 / 3=160 \mathrm{plf}$ Use 2 plies 1-3/4" x 11-7/8"

| Span | 1-3/4" $\times 7-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 4 "$ |  |  |  | 1-3/4" $\times$ 9-1/2" |  |  |  | 1-3/4" $\times 11-1 / 4^{\prime \prime}$ |  |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathrm{L} / 240 \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathrm{L} / 240 \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathbf{L} / 240 \\ \hline \end{array}$ |  | Live Load |  | Total Load |  |  |
|  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 | L/240 |  |  |
| 61 | 494 | 659 | 985 | 1071 | 947 | 1263 |  | 1366 | 1014 | 1353 |  | 1403 | 1553 |  |  | 1662 | 61 |
| 7' | 323 | 430 | 642 | 917 | 630 | 840 |  | 1170 | 677 | 902 |  | 1202 | 1054 | 1406 |  | 1423 | 7' |
| 8' | 221 | 295 | 439 | 802 | 438 | 584 | 872 | 1023 | 471 | 628 | 938 | 1051 | 744 | 992 |  | 1245 | 8' |
| $9^{\prime}$ | 158 | 211 | 313 | 639 | 316 | 421 | 627 | 909 | 340 | 454 | 676 | 933 | 542 | 723 | 1079 | 1105 | 91 |
| $10^{\prime}$ | 116 | 155 | 230 | 516 | 235 | 313 | 465 | 817 | 253 | 337 | 502 | 839 | 406 | 542 | 807 | 994 | 10' |
| $11^{\prime}$ | 88 | 118 | 173 | 426 | 179 | 238 | 353 | 676 | 193 | 257 | 381 | 711 | 311 | 415 | 618 | 903 | 11' |
| 12' | 68 | 91 | 133 | 357 | 139 | 186 | 274 | 567 | 150 | 200 | 296 | 597 | 244 | 325 | 482 | 823 | 12' |
| $13^{\prime}$ | 54 | 72 | 105 | 303 | 110 | 147 | 216 | 482 | 119 | 159 | 234 | 508 | 194 | 259 | 383 | 700 | $13^{\prime}$ |
| $14^{\prime}$ | 43 | 58 | 83 | 261 | 89 | 119 | 173 | 415 | 96 | 128 | 188 | 437 | 157 | 209 | 309 | 602 | 14' |
| $15^{\prime}$ | 35 | 47 | 67 | 227 | 73 | 97 | 141 | 361 | 78 | 105 | 153 | 380 | 128 | 171 | 252 | 524 | $15^{\prime}$ |
| $16^{\prime}$ | - | - | - | - | 60 | 80 | 116 | 316 | 65 | 87 | 125 | 333 | 107 | 142 | 208 | 459 | $16^{\prime}$ |
| $17^{\prime}$ | - | - | - | - | 50 | 67 | 96 | 280 | 54 | 72 | 104 | 294 | 89 | 119 | 173 | 406 | $17{ }^{\prime}$ |
| 18' | - | - | - | - | 42 | 57 | 80 | 249 | 46 | 61 | 87 | 262 | 75 | 101 | 146 | 361 | 18' |
| $19^{\prime}$ | - | - | - | - | 36 | 48 | 68 | 223 | 39 | 52 | 74 | 234 | 64 | 86 | 124 | 324 | 19' |
| $20^{\prime}$ | - | - | - | - | 31 | 41 | 58 | 200 | 33 | 45 | 63 | 211 | 55 | 74 | 105 | 291 | $20^{\prime}$ |
| 21' | - | - | - | - | - | - | - | - | - | - | - | - | 48 | 64 | 91 | 264 | 21' |
| 22' | - | - | - | - | - | - | - | - | - | - | - | - | 42 | 56 | 78 | 239 | 22' |


| Span | 1-3/4" $\times 11-7 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 14$ |  |  |  | 1-3/4" $\times 16$ " |  |  |  | 1-3/4" $\times 18{ }^{\prime \prime}$ |  |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Live Load |  | Total Load <br> L/240 |  | Live Load |  | Total Load <br> L/240 |  | Live Load |  | Total Load |  | Live Load |  | Total Load |  |  |
|  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 | L/240 |  | L/480 | L/360 | L/240 |  |  |
| 10' | 471 | 628 | 937 | 1049 | 735 | 980 |  | 1237 | 1042 | 1390 |  | 1414 | 1404 |  |  | 1591 | 10' |
| 11' | 362 | 483 | 719 | 953 | 569 | 759 |  | 1124 | 813 | 1084 |  | 1285 | 1104 |  |  | 1445 | 11' |
| 12' | 284 | 379 | 562 | 873 | 449 | 599 | 891 | 1030 | 645 | 860 |  | 1177 | 882 | 1176 |  | 1324 | 12' |
| $13^{\prime}$ | 226 | 302 | 447 | 776 | 360 | 480 | 713 | 950 | 520 | 693 | 1032 | 1085 | 714 | 952 |  | 1221 | $13^{\prime}$ |
| $14^{\prime}$ | 183 | 244 | 361 | 668 | 292 | 390 | 578 | 881 | 424 | 566 | 841 | 1007 | 585 | 780 |  | 1133 | $14^{\prime}$ |
| 15' | 150 | 200 | 295 | 581 | 241 | 321 | 475 | 790 | 350 | 467 | 693 | 939 | 485 | 647 | 961 | 1057 | 15' |
| 16' | 125 | 166 | 244 | 509 | 200 | 267 | 394 | 693 | 292 | 390 | 577 | 880 | 406 | 542 | 804 | 990 | 16' |
| $17^{\prime}$ | 104 | 139 | 204 | 450 | 168 | 225 | 330 | 613 | 246 | 329 | 485 | 787 | 343 | 458 | 678 | 931 | 17' |
| 18' | 88 | 118 | 171 | 401 | 143 | 190 | 279 | 546 | 209 | 279 | 411 | 701 | 292 | 390 | 576 | 873 | 18' |
| 19' | 75 | 101 | 145 | 359 | 122 | 163 | 238 | 489 | 179 | 239 | 351 | 628 | 251 | 335 | 493 | 783 | 19' |
| $20^{\prime}$ | 65 | 87 | 124 | 323 | 105 | 140 | 204 | 440 | 155 | 207 | 302 | 566 | 217 | 289 | 425 | 705 | 201 |
| 21' | 56 | 75 | 107 | 292 | 91 | 122 | 176 | 398 | 134 | 179 | 261 | 512 | 189 | 252 | 369 | 638 | 21' |
| 22' | 49 | 65 | 92 | 266 | 80 | 106 | 153 | 362 | 118 | 157 | 227 | 466 | 165 | 220 | 322 | 581 | 22' |
| $23^{\prime}$ | 43 | 57 | 80 | 242 | 70 | 93 | 133 | 331 | 103 | 138 | 199 | 425 | 145 | 194 | 282 | 530 | $23^{\prime}$ |
| 24' | 38 | 51 | 70 | 222 | 62 | 82 | 117 | 303 | 91 | 122 | 175 | 390 | 128 | 171 | 248 | 486 | $24^{\prime}$ |
| $25^{\prime}$ | 33 | 45 | 61 | 204 | 55 | 73 | 103 | 278 | 81 | 108 | 154 | 358 | 114 | 152 | 220 | 447 | $25^{\prime}$ |
| 26' | 30 | 40 | 54 | 188 | 49 | 65 | 91 | 257 | 72 | 96 | 137 | 330 | 102 | 136 | 195 | 412 | $26^{\prime}$ |
| $27^{\prime}$ | - | - | - | - | 43 | 58 | 80 | 237 | 65 | 86 | 121 | 306 | 91 | 122 | 174 | 382 | $27^{\prime}$ |
| 28' | - | - | - | - | 39 | 52 | 71 | 220 | 58 | 77 | 108 | 283 | 82 | 109 | 155 | 354 | 28' |
| 29' | - | - | - | - | 35 | 47 | 64 | 205 | 52 | 70 | 97 | 263 | 74 | 99 | 139 | 329 | 29' |
| 30' | - | - | - | - | 32 | 42 | 57 | 191 | 47 | 63 | 87 | 246 | 67 | 89 | 125 | 307 | 30' |

## DESIGN ASSUMPTIONS:

1. Span is the center-to-center distance of the supports and is valid for simple or equal, continuous span applications.
2. The values in the tables are for uniform loads only.
3. Factored Total Resistance is for standard ( $100 \%$ ) duration and is adjusted to account for the self-weight of the member. The specified dead load shall not exceed the specified live load
4. Live Load Deflection Resistance is limited to $\mathrm{L} / 360$ or $\mathrm{L} / 480$ as noted in the table. Vibration has not been considered
5. Total Deflection Resistance is limited to $L / 240$. Long term deflection (creep) has not been considered
6. These tables assume full lateral support of the compression edge. In lieu of a lateral stability analysis: Members with a depth-towidth ratio not exceeding $6.5: 1$ shall be considered to have full lateral support by direct connection, to the compression edge of the member, of structural wood panel sheathing or by joists spaced not more than $24^{\prime \prime}$ oc. Members with a depth-to-width ratio not exceeding $7.5: 1$ shall also have adequate bridging or blocking installed at an interval not to exceed 8 times the depth of the member Members with a depth-to-width ratio not exceeding 9:1 shall have both edges supported. Other conditions require further analysis by a design professional.
7. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 4

ACTUAL DEFLECTION BASED ON SPAN AND LIMIT

| Span (ft) | L/480 | L/360 | L/240 |
| :---: | :---: | :---: | :---: |
| 10' | 1/4" | 5/16" | 1/2" |
| 12' | 5/16" | 3/8" | 5/8" |
| $14{ }^{\prime}$ | 3/8" | 7/16" | 11/16" |
| $16^{\prime}$ | 3/8" | 9/16" | 13/16" |
| 18' | 7/16" | 5/8" | 7/8" |
| $20^{\prime}$ | 1/2" | 11/16" | $1{ }^{\prime \prime}$ |
| $22^{\prime}$ | 9/16" | 3/4" | 1-1/8" |
| $24^{\prime}$ | 5/8" | 13/16" | 1-3/16" |
| $26^{\prime}$ | 5/8" | 7/8" | 1-5/16" |
| $28^{\prime}$ | 11/16" | 15/16" | 1-3/8" |
| $30^{\prime}$ | 3/4" | $1^{\prime \prime}$ | 1-1/2" |

## ADDITIONAL NOTES:

1. The tabulated resistances represent the capacity of the member in pounds per lineal foot (plf) of length
2. The designer shall check the Factored Total Resistance, the Total Deflection Resistance and the appropriate Live Load Deflection Resistance columns
3. Where the Deflection Resistance is blank, the Factored Total Resistance governs the design.
4. For $1-3 / 4^{\prime \prime}$ thick LVL, depths of $16^{\prime \prime}$ and greater shall be used with a minimum of two plies unless designed specifically as a single ply with proper lateral bracing, such as a marriage beam for each half of a manufactured home before the units are joined
5. The tabulated resistances in the tables are for a single ply of $1-3 / 4$ " LVL. For a $3-1 / 2^{\prime \prime}$ wide member, divide the design loads by 2 to verify the resistance of each ply. For a $5-1 / 4$ " wide member, divide the design loads by 3.
6. The member width shall be properly built up by connecting plies of the same grade of LVL. Refer to the multiple-ply connections on pages $22-23$.
7. Do not use a product where designated "-" without further analysis by a design professional.

## LVL 2900F ${ }_{\mathrm{b}}$-2.0E Uniform Roof Load (PLF) Tables: 1-3/4"

## TO USE:

1. Select the span required. For beams with a pitch of 2:12 or greater, the horizontal span shall be multiplied by the appropriate roof pitch adjustment factor from the table at the bottom of this page.
2. Divide the design loads by the number of plies to verify each ply of the member. Divide the design loads by 2 to verify a $3-1 / 2^{\prime \prime}$ width or by 3 to verify a $5-1 / 4^{\prime \prime}$ width
3. Compare the factored design total load to the Factored Total Resistance column.
4. Compare the unfactored design total load to the Total Load Deflection Resistance
5. Compare the unfactored design live load to the Live Load Deflection Resistance for the appropriate deflection limit. For a live load deflection limit of L/480, compare the unfactored design live load to the L/480 Live Load Deflection Resistance from the Uniform Floor Load Resistance Tables.
6. Select a product that satisfies all three conditions.

NOTE: The serviceability limit states Importance Factor for Snow Load, Is, of 0.9 can be applied to the specified snow loads for evaluation of the deflection resistance. See the example to the right.

## EXAMPLE:

For an 10' horizontal beam span with a pitch of 4:12, select a 2- and 3-ply beam that satisfies an L/240 Snow Load deflection limit for the following specified loads: Snow Load $=720$ plf; Dead Load $=400$ plf
CALCULATE BEAM SPAN: $10^{\prime} \times 1.054=10.54^{\prime} \longrightarrow$ Use $\mathbf{1 1}^{\prime}$

## CALCULATE DESIGN LOADS:

Factored Total Load $=(1.5 \times 720)+(1.25 \times 400)=1580 \mathrm{plf}$
Unfactored Total Load $=(0.9 \times 720)+400=1048 \mathrm{plf}$
Unfactored Snow Load $=0.9 \times 720=648 \mathrm{plf}$

## SOLUTION FOR A 2-PLY BEAM:

Factored Total Load per ply $=1580 / 2=790$ plf Unfactored Total Load per ply $=1048 / 2=524$ plf Unfactored Snow Load per ply = 648/2 = 324 plf
Use 2 plies 1-3/4" x 11-1/4"

SOLUTION FOR A 3-PLY BEAM
Factored Total Load per ply $=1580 / 3=527 \mathrm{plf}$ Unfactored Total Load per ply $=1048 / 3=350$ plf Unfactored Snow Load per ply $=648 / 3=216$ plf Use 3 plies 1-3/4" x 9-1/4"

| Span | 1-3/4" $\times 7-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 2^{\prime \prime}$ |  |  |  | 1-3/4" $\times 11-1 / 4$ " |  |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Snow/L | Load | Total Load |  | Snow/Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \text { L/180 } \\ \hline \end{array}$ |  | Snow/Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \text { L/180 } \\ \hline \end{array}$ |  | Snow/Live Load |  | Total Load |  |  |
|  | L/360 | L/240 | L/180 |  | L/360 | L/240 |  |  | L/360 | L/240 |  |  | L/360 | L/240 | L/180 |  |  |
| 61 | 659 | 989 |  | 1071 | 1263 |  |  | 1366 | 1353 |  |  | 1403 |  |  |  | 1662 | 61 |
| 7' | 430 | 646 | 857 | 917 | 840 |  |  | 1170 | 902 |  |  | 1202 | 1406 |  |  | 1423 | 7' |
| 8' | 295 | 443 | 587 | 802 | 584 | 876 |  | 1023 | 628 | 943 |  | 1051 | 992 |  |  | 1245 | 8' |
| 91 | 211 | 316 | 418 | 639 | 421 | 632 | 838 | 909 | 454 | 681 | 903 | 933 | 723 | 1085 |  | 1105 | 91 |
| 10' | 155 | 233 | 308 | 516 | 313 | 470 | 622 | 817 | 337 | 506 | 671 | 839 | 542 | 813 |  | 994 | 10' |
| 11' | 118 | 177 | 232 | 426 | 238 | 358 | 473 | 676 | 257 | 386 | 510 | 711 | 415 | 623 | 826 | 903 | 11' |
| 12' | 91 | 137 | 179 | 357 | 186 | 279 | 367 | 567 | 200 | 301 | 397 | 597 | 325 | 488 | 645 | 823 | 12' |
| $13^{\prime}$ | 72 | 108 | 141 | 303 | 147 | 221 | 290 | 482 | 159 | 239 | 314 | 508 | 259 | 388 | 513 | 700 | $13^{\prime}$ |
| 14' | 58 | 87 | 113 | 261 | 119 | 178 | 233 | 415 | 128 | 193 | 252 | 437 | 209 | 314 | 413 | 602 | $14^{\prime}$ |
| 15' | 47 | 71 | 91 | 227 | 97 | 146 | 190 | 361 | 105 | 157 | 205 | 380 | 171 | 257 | 338 | 524 | $15^{\prime}$ |
| $16^{\prime}$ | 39 | 59 | 75 | 199 | 80 | 120 | 156 | 316 | 87 | 130 | 169 | 333 | 142 | 214 | 279 | 459 | $16^{\prime}$ |
| $17{ }^{\prime}$ | 32 | 49 | 62 | 175 | 67 | 101 | 130 | 280 | 72 | 109 | 141 | 294 | 119 | 179 | 233 | 406 | $17{ }^{\prime}$ |
| 18' | - | - | - | - | 57 | 85 | 109 | 249 | 61 | 92 | 118 | 262 | 101 | 151 | 196 | 361 | 18' |
| 19' | - | - | - | - | 48 | 72 | 92 | 223 | 52 | 78 | 100 | 234 | 86 | 129 | 167 | 324 | $19^{\prime}$ |
| $20^{\prime}$ | - | - | - | - | 41 | 62 | 78 | 200 | 45 | 67 | 85 | 211 | 74 | 111 | 143 | 291 | 201 |
| 21' | - | - | - | - | 36 | 54 | 67 | 181 | 39 | 58 | 73 | 191 | 64 | 96 | 123 | 264 | 21' |
| 22' | - | - | - | - | 31 | 47 | 58 | 164 | 34 | 51 | 63 | 173 | 56 | 84 | 106 | 239 | 22' |
| Span | 1-3/4" $\times 11-7 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 14$ |  |  |  | 1-3/4" $\times 16^{\prime \prime}$ |  |  |  | 1-3/4" $\times 18{ }^{\prime \prime}$ |  |  |  | Span |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Snow/Live Load |  | Total Load <br> L/180 |  | Snow/Live Load |  | Total Load |  | Snow/Live Load |  | Total Load |  | Snow/Live Load |  | Total Load |  |  |
|  | L/360 | L/240 |  |  | L/360 | L/240 | L/180 |  | L/360 | L/240 | L/180 |  | L/360 | L/240 | L/180 |  |  |
| 10' | 628 | 943 |  | 1049 | 980 |  |  | 1237 | 1390 |  |  | 1414 |  |  |  | 1591 | 10' |
| $11^{\prime}$ | 483 | 725 |  | 953 | 759 |  |  | 1124 | 1084 |  |  | 1285 |  |  |  | 1445 | 11' |
| 12' | 379 | 568 | 752 | 873 | 599 | 898 |  | 1030 | 860 |  |  | 1177 | 1176 |  |  | 1324 | 12' |
| $13^{\prime}$ | 302 | 453 | 598 | 776 | 480 | 720 |  | 950 | 693 | 1040 |  | 1085 | 952 |  |  | 1221 | $13^{\prime}$ |
| 14' | 244 | 367 | 483 | 668 | 390 | 585 | 773 | 881 | 566 | 849 |  | 1007 | 780 |  |  | 1133 | 14' |
| 15' | 200 | 301 | 395 | 581 | 321 | 482 | 635 | 790 | 467 | 701 | 927 | 939 | 647 | 970 |  | 1057 | 15' |
| 16' | 166 | 250 | 327 | 509 | 267 | 401 | 528 | 693 | 390 | 585 | 772 | 880 | 542 | 813 |  | 990 | 16' |
| $17^{\prime}$ | 139 | 209 | 274 | 450 | 225 | 337 | 443 | 613 | 329 | 493 | 650 | 787 | 458 | 687 | 907 | 931 | $17^{\prime}$ |
| 18' | 118 | 177 | 231 | 401 | 190 | 286 | 374 | 546 | 279 | 419 | 551 | 701 | 390 | 585 | 771 | 873 | 18' |
| $19^{\prime}$ | 101 | 151 | 196 | 359 | 163 | 245 | 319 | 489 | 239 | 359 | 471 | 628 | 335 | 502 | 661 | 783 | $19^{\prime}$ |
| $20^{\prime}$ | 87 | 130 | 168 | 323 | 140 | 211 | 274 | 440 | 207 | 310 | 406 | 566 | 289 | 434 | 570 | 705 | 201 |
| 21' | 75 | 113 | 145 | 292 | 122 | 183 | 237 | 398 | 179 | 269 | 351 | 512 | 252 | 378 | 495 | 638 | $21^{\prime}$ |
| 22' | 65 | 98 | 125 | 266 | 106 | 160 | 206 | 362 | 157 | 236 | 306 | 466 | 220 | 331 | 432 | 581 | 22' |
| $23^{\prime}$ | 57 | 86 | 109 | 242 | 93 | 140 | 180 | 331 | 138 | 207 | 268 | 425 | 194 | 291 | 379 | 530 | $23^{\prime}$ |
| 24' | 51 | 76 | 96 | 222 | 82 | 124 | 158 | 303 | 122 | 183 | 236 | 390 | 171 | 257 | 334 | 486 | 24' |
| 25' | 45 | 67 | 84 | 204 | 73 | 110 | 139 | 278 | 108 | 162 | 209 | 358 | 152 | 229 | 296 | 447 | $25^{\prime}$ |
| $26^{\prime}$ | 40 | 60 | 74 | 188 | 65 | 98 | 123 | 257 | 96 | 145 | 185 | 330 | 136 | 204 | 263 | 412 | $26^{\prime}$ |
| $27^{\prime}$ | 36 | 54 | 66 | 174 | 58 | 87 | 110 | 237 | 86 | 130 | 165 | 306 | 122 | 183 | 235 | 382 | $27^{\prime}$ |
| 28' | 32 | 48 | 58 | 161 | 52 | 78 | 98 | 220 | 77 | 116 | 147 | 283 | 109 | 164 | 210 | 354 | $28^{\prime}$ |
| 29' | - | - | - | - | 47 | 71 | 87 | 205 | 70 | 105 | 132 | 263 | 99 | 148 | 189 | 329 | 29' |
| 30' | - | - | - | - | 42 | 64 | 78 | 191 | 63 | 95 | 119 | 246 | 89 | 134 | 170 | 307 | 30' |

## DESIGN ASSUMPTIONS:

1. Span is the center-to-center distance of the supports, along the sloped length of the member and is valid for simple or equal, continuous span applications.
2. The values in the tables are for uniform loads only.
3. Factored Total Resistance is for standard ( $100 \%$ ) duration and is adjusted to account for the self-weight of the member. The specified dead load shall not exceed the specified live load
4. Live Load Deflection Resistance is limited to $\mathrm{L} / 360$ or $\mathrm{L} / 240$ as noted in the table.
5. Total Deflection Resistance is limited to $\mathrm{L} / 180$. Long term deflection (creep) has not been considered
6. These tables assume full lateral support of the compression edge. In lieu of a lateral stability analysis: Members with a depth-to-width ratio not exceeding 6.5:1 shall be considered to have full lateral support by direct connection, to the compression edge of the member, of structural wood panel sheathing or by joists spaced not more than 24 oc. Members with a depth-to-width ratio not exceeding $7.5: 1$ shall also have adequate bridging or blocking installed at an interval not to exceed 8 times the depth of the member. Members with a depth-to-width ratio not exceeding 9:1 shall have both edges supported. Other conditions require further analysis by a design professional.
7. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 4

## ADDITIONAL NOTES:

| PITCH  <br> ADJUSTMENT  |  |
| :---: | :---: |
| Pitch |  |
| Factor |  |
| $2: 12$ | 1.014 |
| $3: 12$ | 1.031 |
| $4: 12$ | 1.054 |
| $5: 12$ | 1.083 |
| $6: 12$ | 1.118 |
| $7: 12$ | 1.158 |
| $8: 12$ | 1.202 |
| $9: 12$ | 1.250 |
| $10: 12$ | 1.302 |
| $1: 12$ | 1.357 |
| $12: 12$ | 1.414 |

1. The tabulated resistances represent the capacity of the member in pounds per lineal foot (pif) of length
2. The designer shall check the Factored Total Resistance, the Total Deflection Resistance and the appropriate Live Load Deflection Resistance columns.
3. For beams with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate pitch adjustment factor from the table above
4. Where the Deflection Resistance is blank, the Factored Total Resistance governs the design.
5. For $1-3 / 4^{\prime \prime}$ thick LVL, depths of 16 " and greater shall be used with a minimum of two plies unless designed specifically as a single ply with proper lateral bracing, such as a marriage beam for each half of a manufactured home before the units are joined.
6. The tabulated resistances in the tables are for a single ply of $1-3 / 4^{\prime \prime}$ LVL. For a $3-1 / 2^{\prime \prime}$ wide member, divide the design loads by 2 to verify the resistance of each ply. For a $5-1 / 4^{\prime \prime}$ wide member, divide the design loads by 3.
7. The member width shall be properly built up by connecting plies of the same grade of LVL. Refer to the multiple-ply connections on pages 22-23.
8. Do not use a product where designated "-" without further analysis by a design professional

SPECIFIED STRENGTHS \& STIFFNESS (PSI)

| Grade | Bending $f_{b}{ }^{4}$ | Modulus of Elasticity $\mathrm{E}^{5}$ (x10 $\left.{ }^{6} \mathrm{PSI}\right)$ | Shear $\mathrm{f}_{\mathrm{v}}$ | Compression |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\underset{\text { (Perpendicular To Grain) }}{\mathbf{f}_{\mathrm{cp}}}$ |
| $2250 \mathrm{~F}_{\mathrm{b}}-2.0 \mathrm{E}$ | 4158 | 1.5 | 530 | 3751 | 1365 |

NOTES:

1. LP® SolidStart ${ }^{\oplus}$ LVL shall be designed for dry-use conditions only. Dry-use applies to products installed in dry, covered and well ventilated interior conditions in which the equivalent average moisture content in lumber will not exceed 15\% nor a maximum of 19\%.
2. The specified strengths and stiffness are for standard load duration. Specified strengths shall be adjusted according to code. Stiffness shall not be adjusted
3. The specified strengths and stiffness are for members supporting loads applied parallel to the wide face ("edge" or "beam" orientation)
4. The specified Bending strength, $f_{b}$, is tabulated for 12 " depth. For depths greater than 12, multiply $f_{b}$ by ( $12 /$ depth $)^{0.143}$. For depths less than 12 ," multiply $f_{b}$ by ( $12 /$ depth $)^{0.111}$ For depths less than 3-1/2," multiply $f_{b}$ by 1.147 .
5. Deflection calculations shall include both bending and shear deformations.

Deflection for a simple span, uniform load: $\Delta=\frac{270 w L^{4}}{E b d^{3}}+\frac{28.8 w L^{2}}{E b d}$ Where: $\Delta=$ deflection (in) $\quad E=$ modulus of elasticity (from table) $w=$ uniform load (plf) $\quad b=$ width (in)
$L=$ design span (ft) $\quad d=$ depth (in)
Equations for other conditions can be found in engineering references.
SECTION PROPERTIES AND FACTORED RESISTANCES

| Depth | Weight <br> ( $\mathrm{lb} / \mathrm{ft}$ ) |  |  |  | Factored Moment ${ }^{2}$ (lb-ft) |  |  |  | Factored Shear (Ib) |  |  |  | Moment of Inertia (in ${ }^{4}$ ) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\prime \prime}$ | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\prime \prime}$ | 1-3/4" | 3-1/2" | 5-1/4" | 7" | 1-3/4" | 3-1/2" | 5-1/4" | $7{ }^{\prime \prime}$ |
| 4-5/8" | 2.3 | 4.6 | 6.9 | 9.3 | 2163 | 4326 | 6489 | 8652 | 2574 | 5148 | 7721 | 10295 | 14 | 29 | 43 | 58 |
| 5-1/2" | 2.8 | 5.5 | 8.3 | 11.0 | 3001 | 6001 | 9002 | 12002 | 3061 | 6122 | 9182 | 12243 | 24 | 49 | 73 | 97 |
| 7-1/4" | 3.6 | 7.3 | 10.9 | 14.5 | 5056 | 10112 | 15169 | 20225 | 4035 | 8069 | 12104 | 16139 | 56 | 111 | 167 | 222 |
| 9-1/4" | 4.6 | 9.3 | 13.9 | 18.5 | 8011 | 16022 | 24032 | 32043 | 5148 | 10295 | 15443 | 20591 | 115 | 231 | 346 | 462 |
| 9-1/2" | 4.8 | 9.5 | 14.3 | 19.0 | 8425 | 16849 | 25274 | 33699 | 5287 | 10574 | 15860 | 21147 | 125 | 250 | 375 | 500 |
| 11-1/4" | 5.6 | 11.3 | 16.9 | 22.5 | 11594 | 23189 | 34783 | 46378 | 6261 | 12521 | 18782 | 25043 | 208 | 415 | 623 | 831 |
| 11-7/8" | 5.9 | 11.9 | 17.8 | 23.8 | 12841 | 25682 | 38524 | 51365 | 6608 | 13217 | 19825 | 26434 | 244 | 488 | 733 | 977 |
| 14" | 7.0 | 14.0 | 21.0 | 28.0 | 17439 | 34878 | 52317 | 69757 | 7791 | 15582 | 23373 | 31164 | 400 | 800 | 1201 | 1601 |

NOTES

1. The Factored Moment and Shear are for standard load duration and shall be adjusted according to code.
2. The tabulated Factored Moment Resistance assumes continuous lateral support of the compression edge. For other conditions, multiply the Factored Moment Resistance by the beam lateral stability factor, $\mathrm{K}_{\mathrm{L}}$, as defined in the CSA 086 .
3. The $3-1 / 2$ ", $5-1 / 4^{\prime \prime}$ and $7^{\prime \prime}$ beam widths listed above can be either a single piece or a combination of widths. For example, a 7 " wide beam may be a single billet beam of 7 ", two plies of $3-1 / 2^{\prime \prime}$, a single $1-3 / 4^{\prime \prime}$ attached to a $5-1 / 4^{\prime \prime}$ billet beam, a $3-1 / 2^{\prime \prime}$ with a $1-3 / 4^{\prime \prime}$ ply attached to each face, or four plies of $1-3 / 44^{\prime \prime}$. Refer to the Connection Assemblies details on page 22 for additional information.
4. The tabulated weight is an estimate and shall only be used for design purposes. Contact LP for actual shipping weights.

## FASTENERS

Refer to pages 22-23 for information on connecting multiple plies and for the equivalent specific gravity for design of nailed and bolted connections

## FACTORED REACTION RESISTANCE (LBS)

| Bearing Length |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | 1-1/2" | 2" | 2-1/2" | 3" | 3-1/2" | 4" | 4-1/2" | $5{ }^{\prime \prime}$ | 5-1/2" | 6" | 6-1/2" | $7{ }^{\prime \prime}$ | 7-1/2" | 8" | 8-1/2" | $9{ }^{\prime \prime}$ | 9-1/2" | 10" | 10-1/2" | 11" | 11-1/2" | 12" |
| 1-3/4" | 2866 | 3822 | 4777 | 5733 | 6688 | 7644 | 8599 | 9555 | 10510 | 11466 | 12421 | 13377 | 14332 | 15288 | 16243 | 17199 | 18154 | 19110 | 20065 | 21021 | 21976 | 22932 |
| 3-1/2" | 5733 | 7644 | 9555 | 11466 | 13377 | 15288 | 17199 | 19110 | 21021 | 22932 | 24843 | 26754 | 28665 | 30576 | 32487 | 34398 | 36309 | 38220 | 40131 | 42042 | 43953 | 45864 |
| 5-1/4" | 8599 | 11466 | 14332 | 17199 | 20065 | 22932 | 25798 | 28665 | 31531 | 34398 | 37264 | 40131 | 42997 | 45864 | 48730 | 51597 | 54463 | 57330 | 60196 | 63063 | 65929 | 68796 |
| $7{ }^{\prime \prime}$ | 11466 | 15288 | 19110 | 22932 | 26754 | 30576 | 34398 | 38220 | 42042 | 45864 | 49686 | 53508 | 57330 | 61152 | 64974 | 68796 | 72618 | 76440 | 80262 | 84084 | 87906 | 91728 |

## NOTES:

1. Tabulated values are based on the factored compression resistance, perpendicular-to-grain, of the LVL. This is suitable for beams bearing on steel or the end-grain of studs.
2. Verify that the support for the beam is structurally adequate to carry the reaction. The compressive resistance, parallel-to-grain, of studs may require more studs than the bearing length above indicates.
3. For beams bearing on wood plates, the required bearing length will increase based on the bearing resistance (compression perpendicular-to-grain) of the species and grade used for the plate material.
4. Verify local code requirements concerning minimum bearing

## FLOOR BEAM QUICK REFERENCE DETAILS (see page 13 for tables)



## TO USE:

1. Select the correct table for the supported floor joist condition (simple or continuous)
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with an 8 ' span carries $15^{\prime}-0$ " simple span joists on each side.
SOLUTION: Using the Simple-Span Floor Joists table with 30'-0" span carried, select either
3-1/2" x 9-1/4" or 5-1/4" x 7-1/4."


CONTINUOUS FLOOR JOISTS (SPECIFIED FLOOR LOADS: 40 PSF LIVE LOAD, 15 PSF DEAD LOAD)

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | 361 | 38' | 40' |
| 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
| 5'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" |
|  | 5-1/4" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 7'-0' | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" |
| 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 9'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" |
| 10'-0" | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 12'-0" | 3-1/2" | 11-7/8" | 11-7/8" | 14" | 14 " | 14 " | 14 " | 14" | 14" | - |  | - |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" | $14 "$ | $14 "$ |
| 14'-0" | 3-1/2" | 14" | 14" | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | 14" | 14" | 14 " | 14" | - | - |
| 16'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | - | - | - | - | - | - | - | - |

SIMPLE-SPAN FLOOR JOISTS (SPECIFIED FLOOR LOADS: 40 PSF LIVE LOAD, 15 PSF DEAD LOAD)

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | $30^{\prime}$ | 32' | 34' | $36^{\prime}$ | $38^{\prime}$ | $40^{\prime}$ |
| 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
| 5'-0' | 3-1/2" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" |
|  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" |
| 6'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" |
| 7'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | $7-1 / 4^{\prime \prime}$ | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | $7-1 / 4^{\prime \prime}$ | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 9'-0' | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" |
| 12'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14" | 14 " | 14 " | 14" |
|  | 5-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
| 14'-0" | 3-1/2" | 14 " | 14 " | 14" | 14" | 14 " | - | - | - | - | - | - |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14" | 14" | 14 " | 14 " | $14^{\prime \prime}$ | 14" |
| 16'-0" | 3-1/2" | 14" | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | 14" | 14" | 14" | 14" | $14^{\prime \prime}$ | - | - | - | - | - |

## NOTES:

1. Use the Continuous Floor Joists table where the floor joists are continuous (multiple span) over the beam. Use the Simple-Span Floor Joists table where the floor joists frame into the side of or end on top of the beam.
2. Span is center-to-center of supports and is valid for simple and equal, continuous beam spans.
3. End supports require $3^{\prime \prime}$ bearing.

Interior supports require $6^{\prime \prime}$ bearing except $7-1 / 2^{\prime \prime}$ is required where bold.
The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 12 for additional information.
4. Deflections are limited to $L / 360$ live load and $L / 240$ total load.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages $22-23$ for connection details.
6. Do not use where marked "-"

## LVL 2250F-1.5E Combined Beam Quick Reference Tables

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a 9' span supports a $28^{\prime}-0$ " span carried for a 20 psf Roof Live load. SOLUTION: Using the correct table for the roof load with $28^{\prime}-01$ span carried, select either $3-1 / 2^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ or 5 -1/4" $\times 9-1 / 4$.'


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | 26' | 28' | 30' | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 7'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" |
|  | 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 11'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | $14 "$ | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  | 12'-0" | 3-1/2" | 11-7/8" | 14" | 14" | 14" | 14" | 14" | 14" | 14" | 14" | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14 "$ | 14" | 14" |
|  | 14'-0" | 3-1/2" | 14" | $14 "$ | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | - | - |
|  | 16'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 14" | 14" | - | - | - | - | - | - | - | - | - |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14 "$ | - | - | - | - | - | - | - | - | - | - |



## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require $3^{\prime \prime}$ bearing except $4-1 / 2^{\prime \prime}$ is required where bold. The end supports for the standard garage door spans of $9^{\prime}-66^{\prime \prime} 16^{\prime}-6$ " and $18^{\prime}-6$ " are limited to $3^{\prime \prime}$ (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 12 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads include 100 plf for an exterior wall and assume a 2' maximum overhang on the roof and an interior support at mid-span of the floor joists.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages $22-23$ for connection details.
6. Do not use where marked "-"

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a 9' span supports a 28'-0" span carried for a 40 psf Roof Live load. SOLUTION: Using the correct table for the roof load with $28^{\prime}-0^{\prime \prime}$ span carried, select either $3-1 / 2^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ or 5 -1/4" $\times 9-1 / 4$.'


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 7'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 9'-6" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | - | - | - |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
|  | 11'-0" | 3-1/2" | 11-7/8" | $14^{\prime \prime}$ | 14" | 14" | 14" | 14" | 14" | - | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | $14^{\prime \prime}$ | 14" | 14" |
|  | 12'-0" | 3-1/2" | 14 " | $14 "$ | $14 "$ | $14 "$ | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | 14" | 14" | 14" | 14" | 14" | $14 "$ | $14 "$ |
|  | 14'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | - | - | - | - | - | - | - |
|  | 16'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | $28^{\prime}$ | 30' | 32' | 34' | 36 ' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 5'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" |
|  | 7'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" |
|  | 9'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 9'-6" | 3-1/2" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" | - | - | - | - | - |
|  |  | 5-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
|  | 10'-0" | 3-1/2" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" | 14" | 14" | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" | 14" |
|  | 11'-0" | 3-1/2" | 14" | 14" | 14" | 14" | 14" | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" | 14" | $14^{\prime \prime}$ |
|  | 12'-0" | 3-1/2" | 14" | 14" | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14" | 14" | $14{ }^{\prime \prime}$ | - | - | - |
|  | 14'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14^{\prime \prime}$ | 14" | - | - | - | - | - | - | - | - | - |
|  | 16'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require $3^{\prime \prime}$ bearing except $4-1 / 2^{\prime \prime}$ is required where bold. The end supports for the standard garage door spans of $9^{\prime}-6$ ", $16^{\prime}-6$ " and $18^{\prime}-6$ " are limited to $3^{\prime \prime}$ (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 12 for additional information.
3. Deflections are limited to $L / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads include 100 plf for an exterior wall and assume a 2' maximum overhang on the roof and an interior support at mid-span of the floor joists.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages $22-23$ for connection details.
6. Do not use where marked "-"

## LVL $2250 \mathrm{~F}_{\mathrm{b}}$-1.5E Roof Beam Quick Reference Tables

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a 9'-6" span supports a $36^{\prime}-0{ }^{\prime \prime \prime}$ span carried for a 30 psf Roof Snow load.
SOLUTION: Using the correct table for the roof load with $36^{\prime}-0^{\prime \prime}$ span carried, select either
$3-1 / 2^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ or $5-1 / 4^{\prime \prime} \times 9-1 / 4$.'


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 6'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 7'-0" | 3-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 9'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 11'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 12'-0" | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 14'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14 " | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" |
|  | 16'-0" | 3-1/2" | 14 " | 14" | 14" | 14" | $14{ }^{\prime \prime}$ | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | 14" | 14" | 14" | 14" | 14" | $14^{\prime \prime}$ |
|  | 16'-6" | 3-1/2" | 14" | $14 "$ | 14" | 14" | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |


| ش | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 6'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 7'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" |
|  |  | 5-1/4" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" |
|  | 9'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 11'-0" | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 12'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | 14 " |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 14'-0" | 3-1/2" | 14 " | 14 " | 14 " | 14" | 14" | 14" | $14{ }^{\prime \prime}$ | 14 " | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | $14 "$ | 14" | 14" | 14" |
|  | 16'-0" | 3-1/2" | 14" | 14" | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-7/8" | $14^{\prime \prime}$ | 14" | 14" | 14" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | - | - | - |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 14" | 14" | 14" | 14" | 14" | $14{ }^{\prime \prime}$ | - | - | - | - | - |

## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require $3^{\prime \prime}$ bearing except $4-1 / 2^{\prime \prime}$ is required where bold.

The end supports for the standard garage door spans of $9^{\prime}-66^{\prime \prime}$ and $16^{\prime}-6$ " are limited to $3^{\prime \prime}$ (two trimmers) on each end.
The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 12 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads assume a 2' maximum overhang on the roof.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners.

Refer to pages 22-23 for connection details.
6. Do not use where marked "-".

## LVL 2250F ${ }_{\mathrm{b}}$-1.5E Roof Beam Quick Reference Tables

## TO USE:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

EXAMPLE: A beam with a 9 '-6" span supports a $36^{\prime}-0$ " span carried for a 50 psf Roof Snow load. SOLUTION: Using the correct table for the roof load with $36^{\prime}-0$ " span carried, select either $3-1 / 2^{\prime \prime} \times 11-7 / 8^{\prime \prime}$ or $5-1 / 4^{\prime \prime} \times 11-1 / 4$.'


|  | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | $30^{\prime}$ | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 7'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" |
|  | 10'-0" | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 11-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 12'-0" | 3-1/2" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | 14" | 14" | 14" | 14" | 14" | 14" | $14 "$ |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ |
|  | 14'-0" | 3-1/2" | 14" | 14" | 14" | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | - |
|  | 16'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 14" | $14 "$ | $14 "$ | 14" | - | - | - | - | - | - | - |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | - | - | - | - | - | - | - | - | - |


| 문 | Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $20^{\prime}$ | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | 36' | 38' | $40^{\prime}$ |
|  | 4'-0" | 3-1/2" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" | 4-5/8" |
|  | 5'-0" | 3-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  |  | 5-1/4" | 4-5/8" | 4-5/8" | 4-5/8" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" | 5-1/2" |
|  | 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 7'-0" | 3-1/2" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  |  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 9'-0" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" |
|  | 9'-6" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14 "$ |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 11-0" | 3-1/2" | 11-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | 14" | 14" | 14" | 14" | 14" | 14" | 14" |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ |
|  | 12'-0" | 3-1/2" | 14" | 14" | 14" | 14" | 14" | 14" | - | - | - | - | - |
|  |  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14{ }^{\prime \prime}$ | 14" | $14^{\prime \prime}$ | 14" | $14^{\prime \prime}$ |
|  | 14'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 14" | 14" | 14" | 14" | 14" | 14" | - | - | - | - | - |
|  | 16'-0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | 14" | - | - | - | - | - | - | - | - | - | - |
|  | 16'-6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  |  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

## NOTES:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require 3 " bearing except $4-1 / 2^{\prime \prime}$ is required where bold.

The end supports for the standard garage door spans of 9 ' -6 " and $16^{\prime}-6$ " are limited to $3^{\prime \prime}$ (two trimmers) on each end.
The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 12 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads assume a $2^{\prime}$ maximum overhang on the roof.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners.

Refer to pages 22-23 for connection details.
6. Do not use where marked "-".

## TO USE:

1. Select the span required.
2. Divide the design loads by the number of plies to verify each ply of the member. Divide the design loads by 2 to verify a $3-1 / 2^{\prime \prime}$ width or by 3 to verify a $5-1 / 4^{\prime \prime}$ width
3. Compare the factored design total load to the Factored Total Resistance column.
4. Compare the unfactored design total load to the Total Load Deflection Resistance.
5. Compare the unfactored design live load to the Live Load Deflection Resistance for the appropriate deflection limit.
6. Select a product that satisfies all three conditions

## EXAMPLE:

For a 10 ' beam span, select a 2- and 3-ply beam that satisfies an L/360 Live Load deflection limit for the following specified loads: Live Load = 480 plf; Dead Load $=180 \mathrm{plf}$

## CALCULATE DESIGN LOADS

Factored Total Load $=(1.5 \times 480)+(1.25 \times 180)=945 \mathrm{plf}$
Unfactored Total Load $=480+180=660$ plf

## SOLUTION FOR A 2-PLY BEAM:

Factored Total Load per ply $=945 / 2=473$ plf Unfactored Total Load per ply $=660 / 2=330$ plf Unfactored Live Load per ply $=480 / 2=240$ plf Use 2 plies 1-3/4" x 9-1/4"

SOLUTION FOR A 3-PLY BEAM:
Factored Total Load per ply $=945 / 3=315 \mathrm{plf}$ Unfactored Total Load per ply $=660 / 3=220$ plf Unfactored Live Load per ply $=480 / 3=160$ plf Use 3 plies 1-3/4" x 7-1/4"

| Span | 1-3/4" $\times 4-5 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 5-1 / 2^{\prime \prime}$ |  |  |  | 1-3/4" $\times 7-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times$ 9-1/4" |  |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathbf{L} / 240 \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline L / 240 \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathrm{L} / 240 \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathrm{L} / 240 \\ \hline \end{array}$ |  |  |
|  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 |  |  |  |
| 4' | 328 | 438 | 655 | 1026 | 525 | 701 | 1048 | 1220 | 1071 | 1428 |  | 1609 | 1913 |  |  | 2053 | 4' |
| $5{ }^{\prime}$ | 176 | 235 | 350 | 689 | 286 | 382 | 570 | 956 | 605 | 806 | 1206 | 1286 | 1127 | 1503 |  | 1641 | $5{ }^{\prime}$ |
| $6{ }^{\prime}$ | 104 | 139 | 207 | 477 | 171 | 229 | 340 | 663 | 371 | 494 | 738 | 1071 | 710 | 947 |  | 1366 | $6{ }^{\prime}$ |
| 7' | 66 | 89 | 131 | 350 | 110 | 147 | 218 | 486 | 242 | 323 | 480 | 820 | 472 | 630 | 940 | 1170 | 7' |
| 8' | 45 | 60 | 88 | 267 | 75 | 100 | 147 | 371 | 166 | 221 | 329 | 627 | 328 | 438 | 653 | 995 | 8' |
| 91 | 32 | 42 | 61 | 210 | 53 | 71 | 103 | 292 | 118 | 158 | 234 | 494 | 237 | 316 | 469 | 785 | 91 |
| 10' | - | - | - | - | 39 | 52 | 75 | 236 | 87 | 116 | 171 | 399 | 176 | 235 | 347 | 635 | 10' |
| 11' | - | - | - | - | - | - | - | - | 66 | 88 | 129 | 329 | 134 | 179 | 264 | 523 | 11' |
| 12' | - | - | - | - | - | - | - | - | 51 | 68 | 99 | 276 | 104 | 139 | 204 | 439 | 12' |
| $13^{1}$ | - | - | - | - | - | - | - | - | 40 | 54 | 77 | 234 | 83 | 110 | 161 | 373 | $13^{\prime}$ |
| $14^{\prime}$ | - | - | - | - | - | - | - | - | 32 | 43 | 62 | 201 | 66 | 89 | 129 | 321 | 14' |
| $15^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 54 | 73 | 104 | 279 | 15' |
| 16' | - | - | - | - | - | - | - | - | - | - | - | - | 45 | 60 | 86 | 244 | $16^{\prime}$ |
| $17{ }^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 37 | 50 | 71 | 215 | $17{ }^{\prime}$ |
| 18' | - | - | - | - | - | - | - | - | - | - | - | - | 32 | 42 | 59 | 192 | 18' |
| 19' | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | 19' |
| 20' | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | 20' |
| Span | 1-3/4" $\times$ 9-1/2" |  |  |  | 1-3/4" $\times 11-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" x 11-7/8" |  |  |  | 1-3/4" $\times 14$ |  |  |  | Span |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \text { L/240 } \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathbf{L} / 240 \\ \hline \end{array}$ |  | Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \text { L/240 } \\ \hline \end{array}$ |  | Live Load |  | Total Load <br> $L / 240$ |  |  |
|  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 |  |  | L/480 | L/360 |  |  |  |
| 8' | 353 | 471 | 702 | 1047 | 558 | 744 | 1110 | 1245 | 643 | 858 | 1281 | 1314 | 981 | 1309 |  | 1549 | 8' |
| 91 | 255 | 340 | 506 | 826 | 406 | 542 | 808 | 1105 | 470 | 627 | 935 | 1167 | 727 | 969 |  | 1376 | 91 |
| 10' | 190 | 253 | 375 | 668 | 304 | 406 | 604 | 920 | 353 | 471 | 701 | 1019 | 551 | 735 | 1096 | 1237 | 10' |
| 11' | 145 | 193 | 285 | 551 | 233 | 311 | 462 | 759 | 271 | 362 | 538 | 841 | 427 | 569 | 847 | 1124 | 11' |
| 12' | 113 | 150 | 221 | 462 | 183 | 244 | 360 | 637 | 213 | 284 | 420 | 705 | 337 | 449 | 667 | 960 | 12' |
| $13^{\prime}$ | 89 | 119 | 174 | 392 | 145 | 194 | 286 | 541 | 170 | 226 | 334 | 600 | 270 | 360 | 533 | 816 | $13^{\prime}$ |
| $14^{\prime}$ | 72 | 96 | 140 | 337 | 117 | 157 | 230 | 466 | 137 | 183 | 269 | 516 | 219 | 292 | 432 | 703 | $14^{\prime}$ |
| $15^{\prime}$ | 59 | 78 | 113 | 293 | 96 | 128 | 187 | 405 | 113 | 150 | 220 | 449 | 180 | 241 | 354 | 611 | 15' |
| $16^{\prime}$ | 49 | 65 | 93 | 257 | 80 | 107 | 154 | 355 | 93 | 125 | 181 | 393 | 150 | 200 | 294 | 536 | 16' |
| 17' | 41 | 54 | 77 | 227 | 67 | 89 | 128 | 313 | 78 | 104 | 151 | 348 | 126 | 168 | 246 | 473 | $17{ }^{\prime}$ |
| 18' | 34 | 46 | 64 | 202 | 56 | 75 | 108 | 279 | 66 | 88 | 127 | 309 | 107 | 143 | 207 | 421 | 18' |
| 19' | - | - | - | - | 48 | 64 | 91 | 249 | 56 | 75 | 107 | 277 | 91 | 122 | 176 | 377 | 19' |
| $20^{\prime}$ | - | - | - | - | 41 | 55 | 78 | 224 | 49 | 65 | 92 | 249 | 79 | 105 | 151 | 340 | 20' |
| $21^{1}$ | - | - | - | - | 36 | 48 | 66 | 203 | 42 | 56 | 79 | 225 | 68 | 91 | 130 | 307 | 21' |
| 22' | - | - | - | - | 31 | 42 | 57 | 184 | 37 | 49 | 68 | 204 | 60 | 80 | 113 | 279 | 22' |
| $23^{\prime}$ | - | - | - | - | - | - | - | - | 32 | 43 | 59 | 186 | 52 | 70 | 98 | 254 | 23' |
| 24' | - | - | - | - | - | - | - | - | - | - | - | - | 46 | 62 | 86 | 233 | 24' |
| $25^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 41 | 55 | 75 | 214 | $25^{\prime}$ |
| $26^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 36 | 49 | 66 | 197 | 26' |
| $27^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 32 | 43 | 58 | 182 | $27^{\prime}$ |
| 28' | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | 28' |

## DESIGN ASSUMPTIONS:

1. Span is the center-to-center distance of the supports and is valid for simple or equal, continuous span applications
2. The values in the tables are for uniform loads only.
3. Factored Total Resistance is for standard ( $100 \%$ ) duration and is adjusted to account for the self-weight of the member. The specified dead load shall not exceed the specified live load.
4. Live Load Deflection Resistance is limited to $\mathrm{L} / 360$ or $\mathrm{L} / 480$ as noted in the table. Vibration has not been considered.
5. Total Deflection Resistance is limited to L/240. Long term deflection (creep) has not been considered.
6. These tables assume full lateral support of the compression edge. In lieu of a lateral stability analysis: Members with a depth-towidth ratio not exceeding $6.5: 1$ shall be considered to have full lateral support by direct connection, to the compression edge of the member, of structural wood panel sheathing or by joists spaced not more than 24" oc. Members with a depth-to-width ratio not exceeding 7.5:1 shall also have adequate bridging or blocking installed at an interval not to exceed 8 times the depth of the member. Members with a depth-to-width ratio not exceeding 9:1 shall have both edges supported. Other conditions require further analysis by a design professional.
7. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 12.

ACTUAL DEFLECTION
BASED ON SPAN AND LIMIT

| Span (ft) | $\mathbf{L} / 480$ | $\mathbf{L} / 360$ | $\mathbf{L} / 240$ |
| :---: | :---: | :---: | :---: |
| $\mathbf{1 0}$ | $1 / 4^{\prime \prime}$ | $5 / 16^{\prime \prime}$ | $1 / 2^{\prime \prime}$ |
| $12^{\prime}$ | $5 / 16^{\prime \prime}$ | $3 / 8^{\prime \prime}$ | $5 / 8^{\prime \prime}$ |
| $14^{\prime}$ | $3 / 8^{\prime \prime}$ | $7 / 16^{\prime \prime}$ | $11 / 16^{\prime \prime}$ |
| $16^{\prime}$ | $3 / 8^{\prime \prime}$ | $9 / 16^{\prime \prime}$ | $13 / 16^{\prime \prime}$ |
| $18^{\prime}$ | $7 / 16^{\prime \prime}$ | $5 / 8^{\prime \prime}$ | $7 / 8^{\prime \prime}$ |
| $20^{\prime}$ | $1 / 2^{\prime \prime}$ | $11 / 16^{\prime \prime}$ | 1 " |
| $22^{\prime}$ | $9 / 16^{\prime \prime}$ | $3 / 4^{\prime \prime}$ | $1-1 / 8^{\prime \prime}$ |
| $24^{\prime}$ | $5 / 8^{\prime \prime}$ | $13 / 16^{\prime \prime}$ | $1-3 / 16^{\prime \prime}$ |
| $26^{\prime}$ | $5 / 8^{\prime \prime}$ | $7 / 8^{\prime \prime}$ | $1-5 / 16^{\prime \prime}$ |
| $28^{\prime}$ | $11 / 16^{\prime \prime}$ | $15 / 16^{\prime \prime}$ | $1-3 / 8^{\prime \prime}$ |
| $\mathbf{3 0}$ | $3 / 4^{\prime \prime}$ | $\mathbf{1 "}^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |

* Deflections rounded to the nearest $1 / 16$."


## ADDITIONAL NOTES:

1. The tabulated resistances represent the capacity of the member in pounds per lineal foot (plf) of length
2. The designer shall check the Factored Total Resistance, the Total Deflection Resistance and the appropriate Live Load Deflection Resistance columns.
3. Where the Deflection Resistance is blank, the Factored Total Resistance governs the design
4. For $1-3 / 4$ " thick LVL, depths of 16 " and greater shall be used with a minimum of two plies unless designed specifically as a single ply with proper lateral bracing, such as a marriage beam for each half of a manufactured home before the units are joined.
5. The tabulated resistances in the tables are for a single ply of $1-3 / 4^{\prime \prime}$ LVL. For a $3-1 / 2^{\prime \prime}$ wide member, divide the design loads by 2 to verify the resistance of each ply. For a $5-1 / 4^{\prime \prime}$ wide member, divide the design loads by 3 .
6. The member width shall be properly built up by connecting plies of the same grade of LVL. Refer to the multiple-ply connections on pages 22-23
7. Do not use a product where designated "-" without further analysis by a design professional.

## LVL 2250Fb-1.5E Uniform Roof Load (PLF) Tables: 1-3/4"

## TO USE:

1. Select the span required. For beams with a pitch of 2:12 or greater, the horizontal span shall be multiplied by the appropriate roof pitch adjustment factor from the table at the bottom of this page.
2. Divide the design loads by the number of plies to verify each ply of the member Divide the design loads by 2 to verify a $3-1 / 2^{\text {" }}$ width or by 3 to verify a $5-1 / 4^{\text {" }}$ width
3. Compare the factored design total load to the Factored Total Resistance column.
4. Compare the unfactored design total load to the Total Load Deflection Resistance
5. Compare the unfactored design live load to the Live Load Deflection Resistance for the appropriate deflection limit. For a live load deflection limit of L/480, compare the unfactored design live load to the L/480 Live Load Deflection Resistance from the Uniform Floor Load Resistance Tables.
6. Select a product that satisfies all three conditions.

NOTE: The serviceability limit states Importance Factor for Snow Load, Is, of 0.9 can be applied to the specified snow loads for evaluation of the deflection resistance. See the example to the right.

## EXAMPLE:

For an 8' horizontal beam span with a pitch of 4:12, select a 2- and 3-ply beam that satisfies an L/240 Snow Load deflection limit for the following specified loads: Snow Load $=720$ plf; Dead Load $=400$ plf
CALCULATE BEAM SPAN: $8^{\prime} \times 1.054=8.43$ ' $\longrightarrow$ Use $\mathbf{9}^{\prime}$

## CALCULATE DESIGN LOADS:

Factored Total Load $=(1.5 \times 720)+(1.25 \times 400)=1580 \mathrm{plf}$
Unfactored Total Load $=(0.9 \times 720)+400=1048 \mathrm{plf}$
Unfactored Snow Load $=0.9 \times 720=648$ plf

SOLUTION FOR A 2-PLY BEAM:
Factored Total Load per ply $=1580 / 2=790$ plf Unfactored Total Load per ply $=1048 / 2=524$ plf Unfactored Snow Load per ply $=648 / 2=324$ plf
Use 2 plies 1-3/4" x 9-1/2"

SOLUTION FOR A 3-PLY BEAM
Factored Total Load per ply $=1580 / 3=527$ plf Unfactored Total Load per ply $=1048 / 3=350$ plf Unfactored Snow Load per ply $=648 / 3=216$ plf Use 3 plies 1-3/4" x 9-1/4"

| Span | 1-3/4" $\times 4-5 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 5-1 / 2^{\prime \prime}$ |  |  |  | 1-3/4" $\times 7-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 4^{\prime \prime}$ |  |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Snow/Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathrm{L} / 180 \\ \hline \end{array}$ |  | Snow/Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathrm{L} / 180 \\ \hline \end{array}$ |  | Snow/Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \mathbf{L} / 180 \\ \hline \end{array}$ |  | Snow/Live Load |  | Total Load |  |  |
|  | L/360 | L/240 |  |  | L/360 | L/240 |  |  | L/360 | L/240 |  |  | L/360 | L/240 | L/180 |  |  |
| 4' | 438 | 657 | 874 | 1026 | 701 | 1051 |  | 1220 | 1428 |  |  | 1609 |  |  |  | 2053 | 4' |
| 5' | 235 | 352 | 467 | 689 | 382 | 573 | 761 | 956 | 806 | 1210 |  | 1286 | 1503 |  |  | 1641 | $5^{\prime}$ |
| $6{ }^{\prime}$ | 139 | 209 | 276 | 477 | 229 | 343 | 455 | 663 | 494 | 742 | 985 | 1071 | 947 |  |  | 1366 | 6' |
| $7{ }^{\prime}$ | 89 | 133 | 176 | 350 | 147 | 221 | 292 | 486 | 323 | 484 | 642 | 820 | 630 | 945 |  | 1170 | $7{ }^{\prime}$ |
| 8' | 60 | 90 | 118 | 267 | 100 | 150 | 197 | 371 | 221 | 332 | 439 | 627 | 438 | 657 | 872 | 995 | 8' |
| 91 | 42 | 64 | 83 | 210 | 71 | 106 | 139 | 292 | 158 | 237 | 313 | 494 | 316 | 474 | 627 | 785 | 9' |
| $10^{\prime}$ | 31 | 47 | 60 | 170 | 52 | 78 | 101 | 236 | 116 | 175 | 230 | 399 | 235 | 352 | 465 | 635 | $10^{\prime}$ |
| 11' | - | - | - | - | 39 | 59 | 76 | 194 | 88 | 133 | 173 | 329 | 179 | 268 | 353 | 523 | 11' |
| $12^{\prime}$ | - | - | - | - | 30 | 45 | 58 | 163 | 68 | 103 | 133 | 276 | 139 | 209 | 274 | 439 | 12' |
| $13^{\prime}$ | - | - | - | - | - | - | - | - | 54 | 81 | 105 | 234 | 110 | 166 | 216 | 373 | $13^{\prime}$ |
| $14{ }^{\prime}$ | - | - | - | - | - | - | - | - | 43 | 65 | 83 | 201 | 89 | 133 | 173 | 321 | 14' |
| $15^{\prime}$ | - | - | - | - | - | - | - | - | 35 | 53 | 67 | 175 | 73 | 109 | 141 | 279 | 15' |
| $16{ }^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 60 | 90 | 116 | 244 | 16' |
| $17{ }^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 50 | 75 | 96 | 215 | 17' |
| 18' | - | - | - | - | - | - | - | - | - | - | - | - | 42 | 64 | 80 | 192 | 18' |
| $19^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 36 | 54 | 68 | 171 | 19' |
| $20^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 31 | 47 | 58 | 154 | 20' |
| Span | 1-3/4" $\times$ 9-1/2" |  |  |  | 1-3/4" $\times 11-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 11-7 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 14^{\prime \prime}$ |  |  |  | Span |
|  | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance | Unfactored Defl. Resistance |  |  | Factored Total Resistance |  |
|  | Snow/Live Load |  | Total Load |  | Snow/Live Load |  | $\begin{array}{\|c\|} \hline \text { Total Load } \\ \hline \text { L/180 } \\ \hline \end{array}$ |  | Snow/Live Load |  | Total Load |  | Snow/Live Load |  | Total Load |  |  |
|  | L/360 | L/240 | L/180 |  | L/360 | L/240 |  |  | L/360 | L/240 | L/180 |  | L/360 | L/240 | L/180 |  |  |
| 8' | 471 | 707 | 938 | 1047 | 744 | 1116 |  | 1245 | 858 | 1287 |  | 1314 | 1309 |  |  | 1549 | 8' |
| 91 | 340 | 510 | 676 | 826 | 542 | 813 | 1079 | 1105 | 627 | 941 |  | 1167 | 969 |  |  | 1376 | $9{ }^{\prime}$ |
| $10^{\prime}$ | 253 | 380 | 502 | 668 | 406 | 609 | 807 | 920 | 471 | 707 | 937 | 1019 | 735 | 1103 |  | 1237 | 10' |
| 11' | 193 | 290 | 381 | 551 | 311 | 467 | 618 | 759 | 362 | 543 | 719 | 841 | 569 | 854 |  | 1124 | 11' |
| 12' | 150 | 226 | 296 | 462 | 244 | 366 | 482 | 637 | 284 | 426 | 562 | 705 | 449 | 674 | 891 | 960 | 12' |
| $13^{\prime}$ | 119 | 179 | 234 | 392 | 194 | 291 | 383 | 541 | 226 | 340 | 447 | 600 | 360 | 540 | 713 | 816 | $13^{\prime}$ |
| $14^{\prime}$ | 96 | 144 | 188 | 337 | 157 | 235 | 309 | 466 | 183 | 275 | 361 | 516 | 292 | 439 | 578 | 703 | 14' |
| $15^{\prime}$ | 78 | 118 | 153 | 293 | 128 | 193 | 252 | 405 | 150 | 226 | 295 | 449 | 241 | 361 | 475 | 611 | $15^{\prime}$ |
| $16{ }^{\prime}$ | 65 | 98 | 125 | 257 | 107 | 160 | 208 | 355 | 125 | 187 | 244 | 393 | 200 | 301 | 394 | 536 | $16^{\prime}$ |
| $17{ }^{\prime}$ | 54 | 82 | 104 | 227 | 89 | 134 | 173 | 313 | 104 | 157 | 204 | 348 | 168 | 253 | 330 | 473 | $17^{\prime}$ |
| 18' | 46 | 69 | 87 | 202 | 75 | 113 | 146 | 279 | 88 | 133 | 171 | 309 | 143 | 214 | 279 | 421 | 18' |
| $19^{\prime}$ | 39 | 59 | 74 | 180 | 64 | 97 | 124 | 249 | 75 | 113 | 145 | 277 | 122 | 183 | 238 | 377 | 19' |
| 20' | 33 | 50 | 63 | 162 | 55 | 83 | 105 | 224 | 65 | 98 | 124 | 249 | 105 | 158 | 204 | 340 | 20' |
| 21' | - | - | - | - | 48 | 72 | 91 | 203 | 56 | 84 | 107 | 225 | 91 | 137 | 176 | 307 | 21' |
| 22' | - | - | - | - | 42 | 63 | 78 | 184 | 49 | 74 | 92 | 204 | 80 | 120 | 153 | 279 | 22' |
| 23' | - | - | - | - | 36 | 55 | 68 | 168 | 43 | 65 | 80 | 186 | 70 | 105 | 133 | 254 | 23' |
| 24' | - | - | - | - | 32 | 48 | 59 | 153 | 38 | 57 | 70 | 170 | 62 | 93 | 117 | 233 | 24' |
| $25^{\prime}$ | - | - | - | - | - | - | - | - | 33 | 50 | 61 | 156 | 55 | 82 | 103 | 214 | 25' |
| $26{ }^{\prime}$ | - | - | - | - | - | - | - | - | 30 | 45 | 54 | 144 | 49 | 73 | 91 | 197 | $26^{\prime}$ |
| $27^{\prime}$ | - | - | - | - | - | - | - | - | - | - | - | - | 43 | 65 | 80 | 182 | $27^{\prime}$ |
| 28' | - | - | - | - | - | - | - | - | - | - | - | - | 39 | 59 | 71 | 169 | 28' |

## DESIGN ASSUMPTIONS:

1. Span is the center-to-center distance of the supports, along the sloped length of the member and is valid for simple or equal, continuous span applications.
2. The values in the tables are for uniform loads only.
3. Factored Total Resistance is for standard ( $100 \%$ ) duration and is adjusted to accountfor the self-weight of the member. The specified dead load shall not exceed the specified live load.
4. Live Load Deflection Resistance is limited to $L / 360$ or $L / 240$ as noted in the table.
5. Total Deflection Resistance is limited to L/180. Long term deflection (creep) has notbeen considered.
6. These tables assume full lateral support of the compression edge. In lieu of a lateral stability analysis: Members with a depth-to-width ratio not exceeding 6.5:1 shall be considered to have full lateral support by direct connection, to the compression edge of the member, of structural wood panel sheathing or by joists spaced not more than $24^{\prime \prime}$ oc. Members with a depth-to-width ratio not exceeding $7.5: 1$ shall also have adequate bridging or blocking installed at an interval not to exceed 8 times the depth of the member. Members with a depth-to-width ratio not exceeding $9: 1$ shall have both edges supported. Other conditions require further analysis by a design professional.
7. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 12.

## ADDITIONAL NOTES:

| PITCH   <br> ADJUSTMENT   <br> Pitch  Factor <br> $2: 12$   <br> 1.014   <br> $3: 12$   1.031 |  |
| :---: | :---: |
| $4: 12$ | 1.054 |
| $5: 12$ | 1.083 |
| $6: 12$ | 1.118 |
| $7: 12$ | 1.158 |
| $8: 12$ | 1.202 |
| $9: 12$ | 1.250 |
| $10: 12$ | 1.302 |
| $11: 12$ | 1.357 |
| $12: 12$ | 1.414 |

. The tabulated resistances represent the capacity of the member in pounds per lineal foot (plf) of length.
2. The designer shall check the Factored Total Resistance, the Total Deflection Resistance and the appropriate Live Load Deflection Resistance columns.
3. For beams with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate pitch adjustment factor from the table above
4. Where the Deflection Resistance is blank, the Factored Total Resistance governs the design.
5. For $1-3 / 4^{\prime \prime}$ thick LVL, depths of 16 " and greater shall be used with a minimum of two plies unless designed specifically as a single ply with proper lateral bracing, such as a marriage beam for each half of a manufactured home before the units are joined.
6. The tabulated resistances in the tables are for a single ply of $1-3 / 4^{\prime \prime}$ LVL. For a $3-1 / 2^{\prime \prime}$ wide member, divide the design loads by 2 to verify the resistance of each ply. For a $5-1 / 4^{\prime \prime}$ wide member, divide the design loads by 3 .
7. The member width shall be properly built up by connecting plies of the same grade of LVL. Refer to the multiple-ply connections on pages $22-23$.
8. Do not use a product where designated "-" without further analysis by a design professional.


## WARNING

## The following conditions are NOT permitted!

DO NOT USE VISUALLY DAMAGED PRODUCTS WITHOUT FIRST CHECKING WITH YOUR LOCAL LP ${ }^{\ominus}$ SolidStart ${ }^{\oplus}$ ENGINEERED WOOD PRODUCTS DISTRIBUTOR OR SALES OFFICE.
(SEE BACK COVER FOR DETAILS.)



## NOTES:

1. These guidelines apply to uniformly loaded beams selected from the Quick Reference Tables or the Uniform Load Tables or designed with LP's design/specification software only. For all other applications, such as beams with concentrated loads, please contact your LP ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.
2. Round holes can be drilled anywhere in "Area $\mathbf{A}^{\text {" provided that: no more than four holes are cut, }}$ with the minimum spacing described in the diagram. The maximum hole size is $1-1 / 2^{\prime \prime}$ for depths up to 9-1/4," and 2" for depths greater than 9-1/4."
3. Rectangular holes are NOT allowed.
4. DO NOT drill holes in cantilevers without prior approval from the project designer.
5. Other hole sizes and configurations MAY be possible with further engineering analysis. For more information, contact your LP SolidStart Engineered Wood Products distributor
6. Up to three $3 / 4$ " holes may be drilled in "Area B" to accommodate wiring and/or water lines These holes shall be at least 12" apart. The holes shall be located in the middle third of the depth, or a minimum of $3^{\prime \prime}$ from the bottom and top of the beam. For beams shallower than $9-1 / 4$ ", locate holes at mid-depth.
7. Protect plumbing holes from moisture.

NOTE: Protect wood from contact with concrete as required by code



SIDE LOADS ARE NOT RECOMMENDED FOR BEAMS OVER 5-1/2" WIDE UNLESS EQUALLY APPLIED TO BOTH FACES See Connection Assemblies for more information

| DETAIL A | DETAIL B | DETAIL C/E | DETAIL D | DETAIL F | DETAIL G | DETAIL H |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MAXIMUM 4" WIDE 2-PLY BEAMS <br> 2" max. ply thickness | MAXIMUM 6" WIDE 3-PLY BEAMS <br> 2" max. ply thickness | MAXIMUM 7-1/4" WIDE 2-PLY BEAMS <br> 2" maximum side member 3-1/2" main member for C 5-1/4" main member for E | MAXIMUM 9-1/4" WIDE 3-PLY BEAMS | MAXIMUM 7" WIDE 3- OR 4-PLY BEAMS | MAXIMUM 7" WIDE 2-PLY BEAMS | MAXIMUM 7" WIDE 2-, 3- OR 4-PLY BEAMS <br> Simpson SDS 1/4" x 6" <br> Simpson SDW 6-3/4" or equal <br> Simpson SDW may be driven from one side. |

FACTORED UNIFORM SIDE-LOAD RESISTANCE (PLF)

| Connection <br> Detail | 2 Rows of Nails <br> at 12" oc | 3 Rows of Nails <br> at 12" oc | 2 Rows of 1/2" <br> Bolts at 24" oc | 2 Rows of 1/2" <br> Bolts at 12" oc |
| :---: | :---: | :---: | :---: | :---: |
| A | 788 | 1182 | 780 | 1560 |
| B | 591 | 887 | 585 | 1170 |
| C | 591 | 887 | 878 | 1755 |
| D | 525 | 788 | 780 | 1560 |
| E | 525 | 788 | 868 | 1736 |
| F | na | na | 520 | 1040 |
| G | na | na | 1560 | 3120 |
| H | Refer to Simpson Strong-Tie ${ }^{\bullet}$ catalog |  |  |  |

NAIL SCHEDULE

| Nail <br> Length <br> (in) | Nail <br> Diameter <br> (in) | Factored Lateral <br> Resistance <br> (lbs) | Nail Size <br> Factor | Shank Type |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{3 - 1 / 2 "}$ | 0.160 | 216 | 1.10 | common wire |
|  | 0.152 | 197 | 1.00 | spiral |
|  | 0.144 | 178 | 0.90 | common wire |
|  | 0.122 | 131 | 0.66 | spiral |
|  | 0.120 | 127 | 0.64 | power-driven ${ }^{13}$ |
|  | 0.144 | 178 | 0.90 | common wire |
|  | 0.122 | 131 | 0.66 | spiral |

## NOTES:

1. When driving nails from each face, alternate every other nail in each row.
2. Unless specifically designed, use $3-1 / 2^{\prime \prime}$ nails for $1-3 / 4^{\prime \prime}$ thick plies. If the nails do not fully penetrate the second ply (main member), then the nails shall be driven from both faces.
3. Use 2 rows of nails for depths to 12 ." Use 3 rows of nails for depths greater than $12, "$ up to 18 ." Use 4 rows of nails for depths greater than 18 ," up to $24 . "$
4. Factored resistances are for standard load duration and shall be adjusted according to code. If the dead load exceeds the live load, the appropriate load duration factor (<1) shall be applied.
5. The Factored Uniform Side-Load Resistance values are the maximum factored load that can be applied to either side of the beam, based on the selected connection detail, and represent loads applied uniformly such as joists supported by hangers spaced 24" oc or less. Connections for discrete point loads may be determined with this table by calculating the equivalent fastener schedule within a 2 ' length centered about the point load. Details $\mathbf{B}$ and $\mathbf{D}$ shall have the back ply connected with a number of nails equal to half that used to connect the front ply - see the Side-Load Connection Example and detail on page 23 . All nail and bolt spacing requirements shall be verified. The full length of the beam shall be connected with the standard connection or with the appropriate side-load connection from this table. The beam shall be designed to support all applied loads.
6. The Factored Uniform Side-Load Resistance for nails is based on $3-1 / 2^{\prime \prime}$ spiral nails for $1-3 / 4^{\prime \prime}$ LVL. For other nail sizes, multiply the Factored Uniform Side-Load Resistance by the Nail Size Factor from the Nail Schedule.
7. The Factored Uniform Side-Load Resistance for bolts is based on ASTM grade A-307, $1 / 2^{\prime \prime} \emptyset$ bolts, for loads applied perpendicular-to-grain (see Fastener Design on page 23).
8. For nails at 8 " oc, multiply resistance by 1.5 . For nails at $6^{\prime \prime}$ oc, multiply resistance by 2 . For four rows of nails, double the two-row resistance.
9. For detail $\mathbf{A}$, or when attaching the first two plies for detail $\mathbf{B}$ (and optionally for details $\mathbf{F}$ and $\mathbf{H}$ - see note 11), the nails may be driven all from one face or alternating from both faces. If the nails do not fully penetrate the second ply, then the nails shall be driven from both faces.
10. For details $\mathbf{C}$ and $\mathbf{E}$, when side-loaded, the larger side-load shall be applied to the thicker ply (main member).
11. For details $\mathbf{F}$ and $\mathbf{H}$, it is permissible to nail the plies together before bolting or driving Simpson SDS or SDW (or equal) screws. Nail two plies together (see note 8) then nail one additional ply to each side.
12. Beams wider than $5-1 / 2^{\prime \prime}$ shall be top-loaded or side-loaded from both sides to prevent rotation. For side loads applied to one side of a beam only, the project designer shall verify torsional capacity or detail the beam to prevent rotation due to any side loads. Consult a design professional for other options.
13. Power-driven nails shall have a yield strength equivalent to common wire nails of the same shank diameter.
14. Other nail, screw or bolt configurations are possible. Refer to the Fastener Design table on page 23 or contact your LP® SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor.

## FASTENER DESIGN

| Equivalent Specific Gravity |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Nails Only |  | Nails and <br> Wood Screws | Bolts and Lag Screws |  |  |
| Withdrawal |  | Dowel Bearing |  | Dowel Bearing (into the face only) |  |
| Edge | Face | Edge | Face | Load Applied <br> Parallel to Grain | Load Applied <br> Perpendicular to Grain |
| 0.46 | 0.50 | 0.50 | 0.50 | 0.46 | 0.50 |

NOTES:

1. The equivalent specific gravity for each connection type listed above is for standard load duration and shall be adjusted according to code.
2. Fastener spacing, end and edge distance shall be as specified by code except for nail spacing as specified below.
3. See details to right for fastener and applied load orientation.

NAIL SPACING REQUIREMENTS

| LVL Ply Thickness | Fastener Orientation ${ }^{4}$ | Nail Size ${ }^{1}$ (common wire or spiral) | Minimum End Distance ${ }^{5}$ | Minimum Nail Spacing per Row ${ }^{5}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Single Row | Multiple Rows ${ }^{3}$ |
| $\geq 1-1 / 2^{\prime \prime}$ | Edge | 2-1/2" | 2-1/2" | $4 "$ | $4^{176}$ |
|  |  | $3^{\prime \prime}$ \& 3-1/4" | 2-1/2" | 4" | $5^{16}$ |
|  |  | 3-1/2" | 3-1/2" | 5" | $6^{16,7}$ |
|  | Face | 2-1/2" | 1-1/2" | $3 "$ | 3" |
|  |  | 3" \& 3-1/4" | 1-1/2" | 3" | 3" |
|  |  | 3-1/2" | 1-1/2" | $5{ }^{\prime \prime}$ | $5{ }^{\prime \prime}$ |

## NOTES:

1. Nails are common wire or spiral nails in accordance with CSA 086.
2. Edge distance shall be such that does not cause splitting.
3. Multiple rows of nails shall be offset at least $1 / 2^{\prime \prime}$ and staggered.
4. Edge orientation refers to nails driven into the narrow edge of the LVL, parallel to the face of the veneers. Face orientation refers to nails driven into the wide face of the LVL, perpendicular to the face of the veneers. (See Fastener \& Load Orientation details above.)
5. Minimum End Distance and Minimum Nail Spacing are tabulated based on common wire nails. For nails with smaller diameters, the spacing and end distance of the common wire nail with the next larger diameter may be used.
6. Minimum nail spacing is tabulated for LVL stamped with plant number 1089. The minimum spacing may be reduced 1" for LVL stamped with plant numbers 1066 and 1071.
7. Minimum nail spacing may be reduced $1^{\prime \prime}$ for LVL stamped with plant number 1089, for thickness of 1-3/4" or greater.

## SIDE-LOAD CONNECTION EXAMPLE



EXAMPLE: Assuming a properly designed 3-ply 14" beam, determine the equivalent connection to support a factored point load of 6970 lb applied to the side of the beam.

## SOLUTION:

1. Determine the equivalent PLF load over the $2^{\prime}$ length by dividing the applied factored load by $2: 6970 \mathrm{lb} / 2^{\prime}=3485 \mathrm{plf}$.
2. Divide the equivalent PLF load by the factored uniform side-load resistance for the appropriate detail. For a 14 " depth, 3 rows of nails are required. For detail B with 3 rows of $3-1 / 2^{\prime \prime}$ spiral nails at 12 " oc: 3485 plf / $887=3.9$.
3. The required total number of nails is: $3.9^{*} 3$ rows of nails @ $12^{\prime \prime}$ oc $=11.7$ nails per foot.
4. Connect the front (loaded) ply with the nailing determined in step 3: drive $123-1 / 2^{\prime \prime}$ spiral nails within $12^{\prime \prime}$ to each side of the point load (a total of 24 nails). Verify nail spacing.
5. Connect the back ply with half the number of nails determined in step 4: drive $63-1 / 2^{\prime \prime}$ nails, from the back, within 12 " to each side of the point load (a total of 12 nails). Verify nail spacing.
6. Connect full length of member with the standard nailing or as required for side loads.
7. Project designer shall detail to prevent rotation of the beam due to the applied side load.

## Handling \& Storage Guidelines

- Warning: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe structures and possible collapse.
- Keep LP® SolidStart ${ }^{\oplus}$ LVL dry. These products are intended to resist the effects of moisture on structural performance from normal construction delays but are not intended for permanent exposure to the weather.
- Unload products carefully, by lifting. Support the bundles to reduce excessive bowing. Individual products should be handled in a manner which prevents physical damage during measuring, cutting, erection, etc.
- Keep products stored in wrapped and strapped bundles, stacked no more than 10 ' high. Support and separate bundles with $2 \times 4$ (or larger) stickers spaced no more than 10 ' apart. Keep stickers in line vertically.
- Product must not be stored in contact with the ground, or have prolonged exposure to the weather.
- Use forklifts and cranes carefully to avoid damaging product.
- Do not use a visually damaged product. Call your local LP SolidStart Engineered Wood Products distributor for assistance when damaged products are encountered.

- For satisfactory performance, LP SolidStart LVL must be used under dry, covered and well-ventilated interior conditions in which the equivalent moisture content in lumber will not exceed 16\%.
- For built-up members, LP SolidStart LVL shall be dry before nailing or bolting to avoid trapping moisture.
- LP SolidStart LVL shall not be used for unintended purposes such as ramps and planks.


## LP SolidStart LVL 2900 $_{\mathrm{b}}$-2.0E

Available in:

- lengths up to 60'
- thicknesses of $1-1 / 2^{\prime \prime}$ and $1-3 / 4^{\prime \prime}$
- billet thicknesses of 3-1/2", 5-1/4" and 7"
. standard depths of 7-1/4," 9-1/4," 9-1/2," 11-1/4," 11-7/8", 14", 16", 18", 20", 22" and 23-7/8"

LP SolidStart LVL 2250F $_{\mathrm{b}}$-1.5E
Available in:

- lengths up to 60'
- thicknesses of 1-3/4"
- billet thicknesses of 3-1/2," 5-1/4" and 7"
- standard depths of 4-3/8", 5-1/2," 7-1/4", 9-1/4", 9-1/2", 11-1/4", 11-7/8", 14" and 16"


## GODE EVALUATION

CCMC evaluation reports can be obtained at www.nrc-cnrc.gc.ca.

A water-resistant coating called SiteCote ${ }^{m}$ is applied to LP LVL for extra weather protection during construction. LP LVL is available without SiteCote ${ }^{T M}$ as a special order.
Contact your local distributor for cost and availability.

## LP SolidStart <br> ENGINEERED WOOD PRODUCTS

BUILDING PRoducts

For more information on the full line of LP ${ }^{\circledR}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products or the nearest distributor, visit our web site at LPCorp.com.
Phone: 1-888-820-0325
E-mail: customer.support@LPCorp.com.
LP SolidStart Engineered Wood Products are manufactured at different locations in the United States and Canada. Please verify availability with the LP SolidStart

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